COMMUNITY PLANNING & DEVELOPMENT

9611 SE 36TH STREET | MERCER ISLAND, WA 98040 PHONE: 206.275.7605 | <u>www.mercerisland.gov</u> Inspection Requests: Online: <u>www.MyBuildingPermit.com</u> VM: 206.275.7730

SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Narrative and Plan Submittal

Instructions: This is a template for a simplified Stormwater Report. This form or an equivalent must accompany your Building Permit Application if the answer is "Yes" to each statement below. If "No" is the answer to one or more of the statements below, a full Drainage Report is required and the project does not qualify for use of the Small Project Stormwater Site Plan/Report template.

Select "yes" or "no" for each statement below. Answer "yes" if the statement accurately describes your project.

Yes	No	Statement
		This project disturbs less than 1 acre and is not part of a larger common plan of development.
		This project converts less than 3/4 acre to lawn or landscape areas.
		This project will create, add, or replace (in any combination) 2,000 square feet or greater, but less than 5,000 square feet, of new plus replaced hard surface OR will have a land disturbing activity of 7,000 square feet or greater OR will result in a net increase of impervious surface of 500 square feet or greater.
		This project will not adversely impact a wetland, stream, water of the state, or change a natural drainage course.

Basic Project Information

Project Name:	
Site Address:	
Total Lot Size:	
Total Proposed Area to be Disturbed (including stockpile area):	sq_ft
Total Volume of Proposed Cut and Fill:	sq ft
Total Proposed New Hard Surface Area:	sq ft
Total Proposed Replaced Hard Surface Area:	sq ft
Total Proposed Converted Pervious Surface Area	
(Native vegetation to lawn or landscape):	sq ft
Net Increase in Impervious Surface:	sq ft



Minimum Requirement #1 : Preparation of Stormwater Site Plan

Written Project Description:

Calculate new or replaced areas by surface type:

Lawn or Landscape Areas:		sq ft	Roof Area:	sq ft
Other Hard Surface Areas:				
Driveway:	sq_ft Patio:		sq ft Sidewalk:	sq ft
Parking Lot:	sq ft Other: _		sq ft	

Attach Drainage Plan

Drainage Plan shall include the following:

- <u>Scaled drawing</u> with slopes, lot lines, any public-right-of-way and any easements, location of each on-site stormwater management BMP selected above and the areas served by them, buildings, roads, parking lots, driveways, landscape features, and areas of disturbed soils to be amended.
- The scaled drawing must be suitable to serve as a recordable document that will be attached to the property deed for each lot that includes on-site BMPs. Document submittal must follow the "Standard Formatting Requirements for Recording Documents" per King County: <u>www.kingcounty.gov/depts/records-licensing/recorders-office/recordingdocuments.aspx</u>
- Identify design details and maintenance instructions for each on-site BMP, and attach them to this Small Project Stormwater Site Plan/Report.



SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Minimum Requirement #2 : Construction Stormwater Pollution Prevention

Complete Section B of this submittal package: Construction Stormwater Pollution Prevention Plan Narrative (SWPPP)

Attach construction SWPPP

Minimum Requirement #3 : Source Control of Pollution

This section contains practices and procedures to reduce the release of pollutants. Provide a description of all known, available and reasonable source control BMPs that will be, or are anticipated to be, used at this location to prevent stormwater from coming into contact with pollutants. Additional BMPs are found in Volume IV of the 2014 Stormwater Management Manual for Western Washington (SWMMWW).

Check the BMPs you will use:

BMP S411 for Landscaping and Lawn/ Vegetation Management Operational practices for sites with landscaping

BMP S421 for Parking and Storage of Vehicles. Public and commercial parking lots can be sources of suspended solids, metals, or toxic hydrocarbons such oils and greases.

BMP S433 for Pools, Spas, Hot Tubs, Fountains Discharge from pools, hot tubs, and fountains can degrade ambient water quality. Routine maintenance activities generate a variety of wastes. Direct disposal of these waters to drainage system and waters of the state are not permitted without prior treatment and approval.

Other BMPs found in Volume IV of SWMMWW applicable to project:



SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Minimum Requirement #4 : Preservation of Natural Drainage Systems

Natural drainage patterns shall be maintained and discharges from the project site shall occur at the natural location, to the maximum extent practicable. All outfalls require energy dissipation.

Choose the option below that best describes your project:



This site has existing drainage systems or outfalls. These items are shown on the Drainage Plan. Include the following items on the Drainage Plan:

- Pipe invert elevations, slopes, cover, and material
- Locations, grades, and direction of flow in ditches and swales, culverts, and pipes

Describe how these systems will be preserved:

This site does not have any existing drainage systems or outfalls.



SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Minimum Requirement #5 : On-site Stormwater Management

All projects meeting the thresholds for this Small Project Stormwater Report shall employ on-site stormwater management BMPs (See Small Project Stormwater Requirements Tip Sheet) to infiltrate, disperse, and retain stormwater runoff on-site to the extent feasible without causing flooding or erosion impacts.

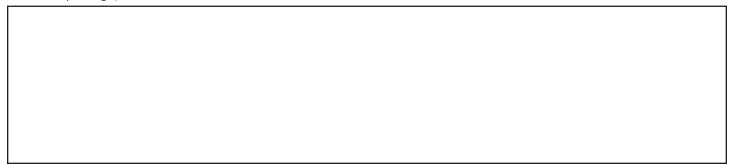
List #1

For each category select the *first* feasible item on the list below. Document your justification for each infeasible BMP in Section C of this submittal package.

Check <u>one</u> option for <u>each category</u> below:

Lawn and Landscape Areas								
	My project does not have Lawn or Landscape areas							
	Post-construction soil quality and depth							
	Post-construction soil quality and depth is infeasi	ble (see Section C of this submittal package)						
Roo	ofs							
	My project does not have <i>Roof</i> areas							
	1. Full dispersion or downspout full infiltration							
	2. Rain garden or bioretention							
	3. Downspout dispersion system	Measured Infiltration Rate: in/ hr						
	4. Perforated stub-out connections	Perforated stub-out connections						
	(applicable if options #1-4 are infeasible and o	site detention authorized by the City Engineer drainage from the site will be discharged to a storm course or there is a capacity constraint in the system)						
	6. No Roof BMP (applicable if options #1-4 are i	nfeasible and on-site detention is not required)						

If #5 or #6 is selected, briefly describe why no Roof BMP is feasible (include detailed information in Section C of this submittal package):



SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Minimum Requirement #5 : On-site Stormwater Management (cont.)

	Other Hard Surfaces (such as driveway, sidewalk, parking lot, patio, etc.)								
	My project does not have Other Hard Surface areas								
		1. Full dispersion	Measured Infiltration Rate: in/ hr						
		2. Permeable pavement, rain gardens, or bioretention	eable pavement, rain gardens, or bioretention						
		3. Sheet flow dispersion or concentrated flow dispersion	eet flow dispersion or concentrated flow dispersion						
		(applicable if options #1-3 are infeasible and drainage fr	detention system or fee-in-lieu of on-site detention authorized by the City Engineer ble if options #1-3 are infeasible and drainage from the site will be discharged to a storm we water system that includes a watercourse or there is a capacity constraint in the system)						
		5. No Other Hard Surface BMP (applicable if options #1-3 are infeasible and on-site detention is not required)							
If #4 or #5	is selec	ted, briefly describe why no Other Hard Surface BMP is fea	sible (include detailed information in						

Flow Control Exempt List

Section C of this submittal package):

Proceed with this list if your project discharges directly to Lake Washington or if findings from a downstream analysis confirm that the downstream system is free of capacity constraints for a minimum of ¼ mile and a maximum of 1 mile.

For flow control exempt discharges, the BMPs listed below for Roofs and Other Hard Surfaces do not need to be evaluated in priority order. You can select any BMP from the lists provided below and do not need to document infeasibility in Section C of this submittal package.

Check one option for each category below:



Lawn and Landscape Areas

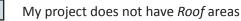
My project does not have Lawn or Landscape areas

Post-construction soil quality and depth

SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Minimum Requirement #5 : On-site Stormwater Management (cont.)

Roofs



Downspout full infiltration



Downspout dispersion system

Perforated stub-out connections



Each item above is infeasible

If "Each item above is infeasible" is selected, briefly describe why no Roof BMP is feasible:

Full Dispersion is not feasible because there is no way to fully disperse roof while still protecting 65% of site. (SWMMWW 2014 BMP T5.30)

Rain Garden or Bioretention is not feasible because the project is in an area where infiltration LIDs are not permitted

Downspout dispersion is not feasible because there is no flowpath of 50 feet or greater between roof downspout to property line (SWMWW 2014 3.1.2)

Perforated stub-out connections are not feasible because the required setbacks cannon be reached (10' from any structure, property line, or sensitive area)(SWMMWW 2014 3.1.1)



Other Hard Surfaces (such as driveway, sidewalk, parking lot, patio, etc.)

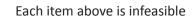
My project does not have Other Hard Surface areas



Sheet flow dispersion



Concentrated flow dispersion



If "Each item above is infeasible" is selected, briefly describe why no Other Hard Surface BMP is feasible:



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Instructions

This is a template for a simplified Construction Stormwater Pollution Prevention Plan ("Construction SWPPP"). If "No" is the answer to one or more of the statements on the first page of Section A of this submittal package, then a full Construction SWPPP is required and the project does not quality for the use of the Small Project Construction SWPPP Narrative template. If the project is less than the thresholds on the first page of Section A of this submittal package, then Minimum Requirement #2 still applies, but this section (Section B) or a full construction SWPPP is not required. You should include your Construction SWPPP in your contract with your builder. A copy of the Construction SWPPP must be located at the construction site or within reasonable access to the site for construction and inspection personnel at all times.

General Information on the Existing Site and Project

Describe the following in the Project Narrative box below (attach additional pages if necessary):

- Nature and purpose of the construction project
- Existing topography, vegetation, and drainage, and building structures
- Adjacent areas, including streams, lakes, wetlands, residential areas, and roads that might be affected by the construction project
- How upstream drainage areas may affect the site
- Downstream drainage leading from the site to the receiving body of water
- Areas on or adjacent to the site that are classified as critical areas
- Critical areas that receive runoff from the site up to one-quarter mile away
- Special requirements and provisions for working near or within critical areas
- Areas on the site that have potential erosion problems

Project Narrative:



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Construction SWPPP Drawings

Refer to the general Drawing Requirements in Stormwater Management Manual for Western Washington (SWMMWW) Volume I, Chapter 3.

Vicinity Map

Provide a map with enough detail to identify the location of the construction site, adjacent roads, and receiving waters.

Sit	те Мар	
Inclu	ude the following (where applicable):	
	Legal description of the property boundaries or an illustration of property lines (including distances) on the drawings.	Final and interim grade contours as appropriate, drainage basins, and the direction of stormwater flow during and upon completion of construction.
	North arrow.	Areas of soil disturbance, including all areas affected by clearing, grading, and excavation.
	Existing structures and roads.	
	Boundaries and identification of different soil types.	Locations where stormwater will discharge to surface waters during and upon completion of construction.
	Areas of potential erosion problems.	Existing unique or valuable vegetation and vegetation to be preserved.
	Any on-site and adjacent surface waters, critical areas, buffers, flood plain boundaries, and Shoreline Management boundaries.	Cut-and-fill slopes indicating top and bottom of slope catch lines.
	Existing contours and drainage basins and the direction of flow for the different drainage areas.	Total cut-and-fill quantities and the method of disposal for excess material.
	Where feasible, contours extend a minimum of 25 feet beyond property lines and extend sufficiently to depict existing conditions.	Stockpile; waste storage; and vehicle storage, maintenance, and washdown areas.
Те	mporary and Permanent BMPs	
Inclu	ude the following on site map (where applicable):	
	Locations for temporary and permanent swales, interceptor trenches, or ditches.	Details for bypassing off-site runoff around disturbed areas.
	Drainage pipes, ditches, or cut-off trenches associated with erosion and sediment control and stormwater management.	Locations of temporary and permanent stormwater treatment and/or flow control best management practices (BMPs).
	Temporary and permanent pipe inverts and minimum slopes and cover.	Details for all structural and nonstructural erosion and sediment control (ESC) BMPs (including, but not limited to, silt fences, construction entrances, sedimentation facilities, etc.)
	Grades, dimensions, and direction of flow in all ditches and swales, culverts, and pipes.	Details for any construction-phase BMPs or techniques used for Low Impact Development (LID) BMP protection.
	Locations and outlets of any dewatering systems.	

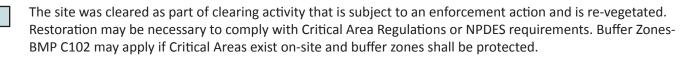


SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 1: Preserve Vegetation / Mark Clearing Limits

The goal of this element is to preserve native vegetation and to clearly show the limits of disturbance.

This element **does not** apply to my project because:



Other Reason / Additional Comments:

If it **does** apply, describe the steps you will take and select the best management practices (BMPs) you will use:

The perimeter of the area to be cleared shall be marked prior to clearing operation with visible flagging, orange plastic barrier fencing and/or orange silt fencing as shown on the SWPPP site map. The total disturbed area shall be less than 7,000 square feet. Vehicles will only be allowed in the areas to be graded, so no compaction of the undeveloped areas will occur.

Additional Comments:

Check the BMPs you will use:

C101 Preserving Natural Vegetation

C102 Buffer Zones





SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 2: Construction Access

The goal of this element is to provide a stabilized construction entrance/exit to prevent or reduce or sediment track out.

This element **<u>does not</u>** *apply to my project because:*



The driveway to the construction area already exists and will be used for construction access. All equipment and vehicles will be restricted to staying on that existing impervious surface.

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

A stabilized construction entrance will be installed prior to any vehicles entering the site, at the location shown on the SWPPP site map.

Additional Comments:

(

C105 Stabilized Construction Entrance / Exit

C106 Wheel Wash



C107 Construction Road / Parking Area Stabilization



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 3: Control Flow Rates

The goal of this element is to construct retention or detention facilities when necessary to protect properties and waterways downstream of development sites from erosion and turbid discharges.

This element **<u>does not</u>** *apply to my project because:*

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Flow rates will be controlled by using SWPPP Element 4 sediment controls and BMP T5.13 Post-Construction Soil Quality and Depth if necessary.



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 4: Sediment Control

The goal of this element is to construct sediment control BMPs that minimize sediment discharges from the site.

This element **<u>does not</u>** apply to my project because:

The site has already been stabilized and re-vegetated.

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Sediment control BMPs shall be placed at the locations shown on the SWPPP site map

Check the BMPs you will use:		
C231 Brush Barrier	C233 Silt Fence	C235 Wattles
C232 Gravel Filter Berm	C234 Vegetated Strip	



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 5: Stabilize Soils

The goal of this element is to stabilize exposed and unworked soils by implementing erosion control BMPs.

This element **<u>does not</u>** apply to my project because:

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Exposed soils shall be worked during the week until they have been stabilized. Soil stockpiles will be located within the disturbed area shown on the SWPPP site map. Soil excavated for the foundation will be backfilled against the foundation and graded to drain away from the building. No soils shall remain exposed and unworked for more than 7 days from May 1 to September 30 or more than 2 days from October 1 to April 30. Once the disturbed landscape areas are graded, the grass areas will be amended using BMP T5.13 Post-Construction Soil Quality and Depth. All stockpiles will be covered with plastic or burlap if left unworked.

Check the BMPs you will us	se:					
C120 Temporary & Permanent Seeding	C12	2 Nets & Blankets	C124 Sodding	C131 Gradient] C2	35 Wattles
C121 Mulching	C12	23 Plastic Covering	C125 Topsoil / Composting	C140 Dust Control		



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 6: Protect Slopes

The goal of this element is to design and construct cut-and-fill slopes in a manner to minimize erosion.

This element **<u>does not</u>** *apply to my project because:*



No cut slopes over 4 feet high or slopes steeper than 2 feet horizontal to 1 foot vertical, and no fill slopes over 4 feet high will exceed 3 feet horizontal to 1 foot vertical. Therefore, there is no requirement for additional engineered slope protection.

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Additional Comments:

r			
Check	k the BMPs you will use:		
	C120 Temporary & Permanent Seeding	C205 Subsurface Drains	C207 Check Dams
	C204 Pipe Slope Drains	C206 Level Spreader	C208 Triangular Silt Dike

(Geotextile-Encased Check Dam)



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 7: Protect Permanent Drain Inlets

The goal of this element is to protect storm drain inlets during construction to prevent stormwater runoff from entering the conveyance system without being filtered or treated.

This element **does not** apply to my project because:

The site has open ditches in the right-of-way or private road right-of-way.

There are no catch basins on or near the site.

Other Reason / Additional Comments:

If it **does** apply, describe the steps you will take and select the BMPs you will use:

Catch basins on the site or immediately off site in the right-of-way are shown on the SWPPP site map. Storm drain inlet protection shall be installed.

Additional Comments:

Check the BMPs you will use:



C220 Storm Drain Inlet Protection



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 8: Stabilize Channels and Outlets

The goal of this element is to design, construct, and stabilize on-site conveyance channels to prevent erosion from entering existing stormwater outfalls and conveyance systems.

This element **<u>does not</u>** *apply to my project because:*

Construction will occur during the dry weather. No storm drainage channels or ditches shall be constructed either temporary or permanent. A small swale shall be graded to convey yard drainage around the structure using a shallow slope; it shall be seeded after grading and stabilized.

Other Reason / Additional Comments:

If it **does** apply, describe the steps you will take and select the BMPs you will use:

A wattle shall be placed at the end of the swale to prevent erosion at the outlet of the swale.

Additional Comments:

Check the BMPs you will use:

C202 Channel Lining

C207 Check Dams







SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 9: Control Pollutants

The goal of this element is to design, install, implement and maintain BMPs to minimize the discharge of pollutants from material storage areas, fuel handling, equipment cleaning, management of waste materials, etc.

This element **<u>does not</u>** *apply to my project because:*

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Any and all pollutants, chemicals, liquid products and other materials that have the potential to pose a threat to human health or the environment will be covered, contained, and protected from vandalism. All such products shall be kept under cover in a secure location on-site. Concrete handling shall follow BMP C151.

Additional Comments:

C151 Concrete Handling C152 Sawcutting and Surfacing Pollution Prevention

C153 Material Delivery, Storage, and Containment

C154 Concrete Washout Area



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 10: Control De-watering

The goal of this element is to handle turbid or contaminated dewatering water separately from stormwater.

This element **<u>does not</u>** apply to my project because:

No dewatering of the site is anticipated.

Other Reason / Additional Comments:

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

Additional Comments:

Check the BMPs you will use:

C203 Water Bars

C236 Vegetated Filtration

C206 Level Spreader



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 11: Maintain Best Management Practices

The goal of this element is to maintain and repair all temporary and permanent erosion and sediment control BMPs to assure continued performance.

Describe the steps you will take:



Best Management Practices or BMPs shall be inspected and maintained during construction and removed within 30 days after the City Inspector or Engineer determines that the site is stabilized, provided that they may be removed when they are no longer needed.

Element 12: Manage the Project

The goal of this element is to ensure that the construction SWPPP is properly coordinated and that all BMPs are deployed at the proper time to achieve full compliance with City regulations throughout the project.

If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:

The Construction SWPPP will be implemented at all times. The applicable erosion control BMPs will be implemented in the following sequence:

1. Mark clearing limits

- 2. Install stabilized construction entrance
- 3. Install protection for existing drainage systems and permanent drain inlets
- 4. Establish staging areas for storage and handling polluted material and BMPs
- 5. Install sediment control BMPs
- 6. Grade and install stabilization measures for disturbed areas
- 7. Maintain BMPs until site stabilization, at which time they may be removed



SECTION B: SMALL PROJECT CONSTRUCTION SWPPP NARRATIVE

Element 13: Protect Low Impact Development BMPs

The goal of this element is to protect on-site stormwater management BMPs (also known as "Low Impact Development BMPs") from siltation and compaction during construction. On-site stormwater management BMPs used for runoff from roofs and other hard surfaces include: full dispersion, roof downspout full infiltration or dispersion systems, perforated stubout connections, rain gardens, bioretention systems, permeable pavement, sheetflow dispersion, and concentrated flow dispersion. Methods for protecting on-site stormwater management BMPs include sequencing the construction to install these BMPs at the latter part of the construction grading operations, excluding equipment from the BMPs and the associated areas, and using the erosion and sedimentation control BMPs listed below.

Describe the construction sequencing you will use:

Additional Comments:

Select the BMPs you will use:

C102 Buffer Zone

C103 High Visibility Fence



C231 Brush Barrier

C233 Silt Fence

C234 Vegetated Strip



SECTION C: INFEASIBILITY CRITERIA

Minimum Requirement #5 (On-Site Stormwater Management)

The following tables summarize infeasibility criteria that can be used to justify not using various on-site stormwater management best management practices (BMPs) for consideration for Minimum Requirement #5. This information is also included under the detailed descriptions of each BMP in the 2014 Stormwater Management Manual for Western Washington (Stormwater Manual), but is provided here in this worksheet for additional clarity and efficiency. Where any inconsistencies or lack of clarity exists, the requirements in the main text of the Stormwater Manual shall be applied. If a project is limited by one or more of the infeasibility criteria specified below, but an applicant is interested in implementing a specific BMP, a functionally equivalent design may be submitted to the City for review and approval. Evaluate the feasibility of the BMPs in priority order based on List #1 or #2 (Small Project Stormwater Requirements Tip Sheet and Stormwater Manual). Select the first BMP that is considered feasible for each surface type. Document the infeasibility (narrative description and rationale) for each BMP that was not selected. Only one infeasibility criterion needs to be selected for a BMP before evaluating the next BMP on the list. Attach additional pages for supporting information if necessary.

Note: If your project discharges directly to Lake Washington (flow control exempt) or a downstream analysis confirms that the downstream system is free of capacity constraints for a minimum of ¼ mile and a maximum of 1 mile, then you do not need to complete this worksheet, but should still refer to the infeasibility criteria when selecting BMPs.

	Lawn and Landscaped Areas	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Post-construction Soil Quality and Depth	 Siting and design criteria provided in BMP T5.13 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. Lawn and landscape area is on till slopes greater than 33 percent. 	
List #1 and #2		
	Roofs	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Full Dispersion List #1 and #2	 Site setbacks and design criteria provided in BMP T5.30 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A 65 to 10 ratio of forested or native vegetation area to impervious area cannot be achieved. A minimum forested or native vegetation flowpath length of 100 feet 	
	 (25 feet for sheet flow from a non-native pervious surface) cannot be achieved. Evaluation of infiltration is not required per the Infiltration 	
Downspout Full Infiltration	Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards.	
List #1 and #2	Site setbacks and design criteria provided in BMP T5.10A (Stormwater Manual Volume III, Section 3.1.1) cannot be achieved. The lot(s) or site does not have out-wash or loam soils.	
	There is not at least 3 feet or more of permeable soil from the proposed final grade to the seasonal high groundwater table or other impermeable layer.	
	There is not at least 1 foot or more of permeable soil from the proposed bottom of the infiltration system to the seasonal high groundwater table or other impermeable layer.	



	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	Note: Criteria with setback distances are as measured from the bottom edge of the bioretention soil mix.	
	Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist):	
	Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or down-gradient flooding.	
	Within an area whose ground water drains into an erosion hazard, or landslide hazard area.	
Bioretention or Rain Gardens	Where the only area available for siting would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, pre-existing structures, or pre-existing road or parking lot surfaces.	
List #1 (both) and List #2 (bioretention only)	Where the only area available for siting does not allow for a safe overflow pathway to stormwater drainage system or private storm sewer system.	
	Where there is a lack of usable space for bioretention areas at re- development sites, or where there is insufficient space within the existing public right-of-way on public road projects.	
	Where infiltrating water would threaten existing below grade basements.	
	Where infiltrating water would threaten shoreline structures such as bulkheads.	
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation):	
	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards	
	Within setback provided for BMP T7.30 (Stormwater Manual Volume V, Section 7.4)	
	Where they are not compatible with surrounding drainage system as determined by the city (e.g., project drains to an existing stormwater collection system whose elevation or location precludes connection to a properly functioning bioretention area).	



	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Bioretention or Rain Gardens (cont.)	 The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where land for bioretention is within an erosion hazard, or landslide hazard area (as defined by MICC 19.07.060). Where the site cannot be reasonably designed to locate bioretention areas on slopes less than 8 percent. NO SPACE BETWEEN STRUCTURES AND EXISTING TREE ROOTS. Within 50 feet from the top of slopes that are greater than 20 percent and over 10 feet of vertical relief. For properties with known soil or groundwater contamination (typically federal Superfund sites or state cleanup sites under the Model Toxics Control Act [MTCA]): Within 100 feet of an area known to have deep soil contamination. Where groundwater modeling indicates infiltration will likely increase or change the direction of the migration of pollutants in the groundwater. Wherever surface soils have been found to be contaminated unless those soils are removed within 10 horizontal feet from the infiltration area. Any area where these facilities are prohibited by an approved cleanup plan under the state MTCA or Federal Superfund Law, or an environmental covenant under Chapter 64.70 RCW. Within 100 feet of a closed or active landfill. Within 10 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is 1,100 gallons or less. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface. 	



	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Bioretention or Rain Gardens (cont.)	 The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where field testing indicates potential bioretention/rain garden sites have a measured (a.k.a., initial) native soil saturated hydraulic conductivity less than 0.30 inches per hour. A small-scale or large-scale PIT in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to demonstrate infeasibility of bioretention areas. If the measured native soil infituration rate is less than 0.30 in/hour, bioretention/rain garden BMPs are not required to be evaluated as an option in List #1 or List #2. In these slow draining soils, a bioretention area with an underdrain may be used to treat pollution-generating surfaces to help meet Minimum Requirement #6, Runoff Treatment. If the underdrain is elevated within a base course of gravel, it will also provide some modest flow reduction benefit that will help achieve Minimum Requirement #7. Where the minimum vertical separation of 3 feet to the seasonal high groundwater elevation or other impermeable layer would not be achieved below bioretention that would serve a drainage area that exceeds the following thresholds (and cannot reasonably be broken down into amounts smaller than indicated): o 5,000 square feet of pollution-generating impervious surface (PGIS) o 10,000 square feet of impervious area o .0.75 acres of lawn and landscape. Where the minimum vertical separation of 1 foot to the seasonal high groundwater or other impermeable layer would not be achieved below bioretention that would serve a drainage area less than the above thresholds. Within 100 feet of a drinking water well, or a spring used for drinking water supply. Within 10 feet of small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks fro	



	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Downspout Dispersion Systems List #1 and #2	 Site setbacks and design criteria provided in BMP T5.10B (Stormwater Manual Volume III, Section 3.1.2) cannot be achieved. For splash blocks, a vegetated flowpath at least 50 feet in length from the downspout to the downstream property line, structure, stream, wetland, slope over 15 percent, or other impervious surface is not feasible. For trenches, a vegetated flowpath of at least 25 feet in between the outlet of the trench and any property line, structure, stream, wetland, or impervious surface is not feasible. A vegetated flowpath of at least 50 feet between the outlet of the trench and any slope steeper than 15 percent is not feasible. 	
Perforated Stub-Out Connections List #1 and #2	 Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards For sites with septic systems, the only location available for the perforated portion of the pipe is located up-gradient of the drainfield primary and reserve areas. This requirement can be waived if site topography will clearly prohibit flows from intersecting the drainfield or where site conditions (soil permeability, distance between systems, etc.) indicate that this is unnecessary. Site setbacks and design criteria provided in BMP T5.10C (Stormwater Manual Volume III, Section 3.1.3) cannot be achieved. There is not at least 1 foot of permeable soil from the proposed bottom (final grade) of the perforated stub-out connection trench to the highest estimated groundwater table or other impermeable layer. The only location available for the perforated stub-out connection is under impervious or heavily compacted soils. 	
On-site Detention List #1 and #2	 Project discharges directly to Lake Washington. Findings from a 1/4 mile downstream analysis confirm that the downstream system is free of capacity constraints. Site setbacks and design criteria provided in the Stormwater Manual (Volume III, Section 3.2.2) cannot be achieved. 	



	Other Hard Surfaces	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Full Dispersion List #1 and #2	 Site setbacks and design criteria provided in BMP T5.30 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A 65 to 10 ratio of forested or native vegetation area to impervious area cannot be achieved. A minimum forested or native vegetation flowpath length of 100 feet (25 feet for sheet flow from a non-native pervious surface) cannot be achieved. 	
Permeable Pavement List #1 and #2	 Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist): Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or downgradient flooding. Within an area whose ground water drains into an erosion hazard, or landslide hazard area. Where infiltrating and ponded water below the new permeable pavement area would compromise adjacent impervious pavements. Where infiltrating water below a new permeable pavement area would threaten existing below grade basements. Where infiltrating water would threaten shoreline structures such as bulkheads. Down slope of steep, erosion prone areas that are likely to deliver sediment. Where fill soils are used that can become unstable when saturated. Excessively steep slopes where water within the aggregate base layer or at the subgrade surface cannot be controlled by detention structures and may cause erosion and structural failure, or where surface runoff velocities may preclude adequate infiltration at the pavement surface. Where installation of permeable pavement would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, or pre-existing road subgrades. 	



	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation):	
	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards	
	Within an area designated as an erosion hazard, or landslide hazard.	
	Within 50 feet from the top of slopes that are greater than 20 percent.	
	For properties with known soil or groundwater contamination (typically federal Superfund sites or state cleanup sites under MTCA):	
	 Within 100 feet of an area known to have deep soil contamination. 	
Permeable Pavement (cont.)	 Where groundwater modeling indicates infiltration will likely increase or change the direction of the migration of pollutants in the groundwater. 	
(cont.)	 Wherever surface soils have been found to be contaminated unless those soils are removed within 10 horizontal feet from the infiltration area. 	
	 Any area where these facilities are prohibited by an approved cleanup plan under the state MTCA or Federal Superfund Law, or an environmental covenant under Chapter 64.70 RCW. 	
	Within 100 feet of a closed or active landfill.	
	Within 100 feet of a drinking water well, or a spring used for drinking water supply, if the pavement is a pollution-generating surface.	
	Within 10 feet of a small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks from a "large on-site sewage disposal system," see Chapter 246-272B WAC.	
	Within 10 feet of any underground storage tank and connecting underground pipes, regardless of tank size. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface.	
	At multi-level parking garages, and over culverts and bridges.	
	Where the site design cannot avoid putting pavement in areas likely to have long-term excessive sediment deposition after construction (e.g., construction and landscaping material yards).	



	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
••	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where the site cannot reasonably be designed to have: Porous asphalt surface < 5% slope Pervious concrete surface < 10% slope Permeable interlocking concrete pavement surface < 12% slope Grid systems < 6-12% slope (check with manufacturer and local supplier to confirm maximum slope) Where the subgrade soils below a pollution-generating permeable pavement (e.g., road or parking lot) do not meet the soil suitability criteria for providing treatment. See soil suitability criteria for treatment in the Stormwater Manual Volume III, Section 3.3.7. Note: In these instances, the city may approve installation of a 6 inch sand filter layer meeting city specifications for treatment as a condition of construction. Where underlying soils are unsuitable for supporting traffic loads when saturated. Soils meeting a California Bearing Ratio of 5 percent are considered suitable for residential access roads. Where replacing existing impervious surfaces unless the existing surface is a non-pollution generating surface over an outwash soil with a saturated hydraulic conductivity of 4 inches per hour or greater. Where appropriate field testing indicates soils have a measured (a.k.a., initial) subgrade soil saturated hydraulic conductivity less than 0.3 inches per hour. Only small-scale PIT or large-scale PIT methods in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to evaluate infeasibility of permeable pavement areas. (Note: In these instances, unless other infeasibility criterions apply, roads and parking lots may be built with an un	



	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Permeable Pavement (cont.)	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): At sites defined as "high-use sites" (refer to the Glossary in the Stormwater Manual Volume I). In areas with "industrial activity" as identified in 40 CFR 122.26(b)(14). Where the risk of concentrated pollutant spills is more likely such as gas stations, truck stops, and industrial chemical storage sites.	
	 Where routine, heavy applications of sand occur in frequent snow zones to maintain traction during weeks of snow and ice accumulation. Where the seasonal high groundwater or an underlying impermeable/ low permeable layer would create saturated conditions within 1 foot of the bottom of the lowest gravel base course. 	
Bioretention or Rain Gardens List #1 (both) and List #2 (bioretention only)	 Note: Criteria with setback distances are as measured from the bottom edge of the bioretention soil mix. Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist): Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or down-gradient flooding. Within an area whose ground water drains into an erosion hazard, or landslide hazard area. Where the only area available for siting would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, pre-existing structures, or pre-existing road or parking lot surfaces. Where the only area available for siting does not allow for a safe overflow pathway to stormwater drainage system or private storm sewer system. Where there is a lack of usable space for bioretention areas at redevelopment sites, or where there is insufficient space within the existing public right-of-way on public road projects. Where infiltrating water would threaten existing below grade basements. Where infiltrating water would threaten shoreline structures such as bulkheads. 	



	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Applicable		and Rationale for Each
	1,100 gallons or less. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface.	



	Other Hard Surfaces (cont.)		
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected	
Applicable	Infeasibility Criteria The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Within 100 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is greater than 1,100 gallons. Where field testing indicates potential bioretention/rain garden sites have a measured (a.k.a., initial) native soil saturated hydraulic conductivity less than 0.30 inches per hour. A small-scale or large-scale PIT in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to demonstrate infeasibility of bioretention areas. If the measured native soil infiltration rate is less than 0.30 in/hour, bioretention/rain garden BMPs are not required to be evaluated as an option in List #1 or List #2. In these slow draining soils, a bioretention area with an underdrain may be used to treat pollution-generating surfaces to help meet Minimum Requirement #6, Runoff Treatment. If the underdrain is elevated within a base course of gravel, it will also provide some modest flow reduction benefit that will help achieve Minimum Requirement #7. Where the minimum vertical separation of 3 feet to the seasonal high groundwater elevation or other impermeable layer would not be achieved below bioretention that would serve a drainage area that exceeds the following thresholds (and cannot reasonably be broken down into amounts smaller than indicated):	and Rationale for Each	
	 o 5,000 square feet of pollution-generating impervious surface (PGIS) o 10,000 square feet of impervious area o 0.75 acres of lawn and landscape. Where the minimum vertical separation of 1 foot to the seasonal high groundwater or other impermeable layer would not be achieved below bioretention that would serve a drainage area less than the above thresholds Within 100 feet of a drinking water well, or a spring used for drinking water supply. Within 10 feet of small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks from a "large on-site sewage disposal system," see Chapter 246-272B WAC. 		



	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Sheet Flow Dispersion List #1 and #2	 Site setbacks and design criteria provided in BMP T5.12 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. Positive drainage for sheet flow runoff cannot be achieved. Area to be dispersed (e.g., driveway, patio) cannot be graded to have less than a 15 percent slope. For flat to moderately sloped areas, at least a 10 foot-wide vegetation buffer for dispersion of the adjacent 20 feet of contributing surface cannot be achieved. For variably sloped areas, at least a 25 foot vegetated flowpath between berms cannot be achieved. 	
Concentrated Flow Dispersion List #1 and #2	 Site setbacks and design criteria provided in BMP T5.11 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A minimum 3 foot length of rock pad and 50 foot flowpath OR a dispersion trench and 25 foot flowpath for every 700 square feet of drainage area followed with applicable setbacks cannot be achieved. More than 700 square feet drainage area drains to any dispersion device. 	
On-site Detention List #1 and #2	 Project discharges directly to Lake Washington. Findings from a 1/4 mile downstream analysis confirm that the downstream system is free of capacity constraints. Site setbacks and design criteria provided in the Stormwater Manual (Volume III, Section 3.2.2) cannot be achieved. 	



SECTION D: POST-CONSTRUCTION SOIL MANAGEMENT

Attachments Required (Check off required items that are attached)
Site Plan showing, to scale:
Areas of undisturbed native vegetation (no amendment required)
New planting beds (amendment required)
New turf areas (amendment required)
Type of soil improvement proposed for each area
Soil test results (required if proposing custom amendment rates)
Product test results for proposed amendments

Total Amendment / Topsoil / Mulch for All Areas

Calculate the quantities needed for the entire site based on all of the areas identified on the Site Plan and the calculations on the following page(s):

Product	Total Quantity (CY)	Test Results	
Product #1:	CY	% organic matter C:N ratio "Stable"?yesno	
Product #2:	CY	% organic matter C:N ratio "Stable"? yes no	
Product #3:	CY	% organic matter C:N ratio "Stable"? yes no	



SECTION D: POST-CONSTRUCTION SOIL MANAGEMENT

Amendment / Topsoil / Mulch by Area

For each identified area on your Site Plan, provide the following information:

Area # _____ (should match identified Area # on Site Plan)

(Use additional sheets if necessary)

Planting type:

Turf Planting Beds

H

Undisturbed native vegetation

Other: _____

Pre-Approved Amend	dment Method
---------------------------	--------------

	Amend with compost	Turf: SF x 5.4 CY ÷ 1,000 SF =CY Planting beds: SF x 9.3 CY ÷ 1,000 SF=CY Total Quantity =CY Scarification depth: 8 inches	Product:	
	Stockpile and amend	Turf: SF x 5.4 CY ÷ 1,000 SF =CY Planting beds: SF x 9.3 CY ÷ 1,000 SF=CY Total Quantity =CY Scarification depth: 8 inches	Product:	
	Topsoil import	Turf: SF x 18.6 CY÷1,000 SF =CY Planting beds: SF x 18.6 CY ÷ 1,000 SF=CY Total Quantity =CY Scarification depth: 6 inches	Product:	
Cus	stom Amendn	nent		
	Amend with compost	Attach information on bulk density, percent organic matter, moisture content, C:N ratio, and heavy metals analysis to support custom amendment rate and scarification depth. Total Quantity =CY Scarification depth:inches	Product:	
	Stockpile and amend	Attach information on bulk density, percent organic matter, moisture content, C:N ratio, and heavy metals analysis to support custom amendment rate and scarification depth. Total Quantity =CY Scarification depth:inches	Product:	
Mulch				
	Amend with compost	Planting beds: SF x 12.4 CY ÷ 1,000 SF=CY Total Quantity =CY	Product:	
	Stockpile and amend	Planting beds: SF x 12.4 CY ÷ 1,000 SF=CY Total Quantity =CY	Product:	
	Topsoil import	Planting beds: SF x 12.4 CY ÷ 1,000 SF=CY Total Quantity =CY	Product:	

CY = cubic yards, C:N = Carbon:Nitrogen



CITY OF MERCER ISLAND SECTION E: SIGNATURE PAGE

Project Engineer's Certification for Section B

For Stormwater Site Plans with engineered elements, the Construction SWPPP is stamped by a professional engineer licensed in the State of Washington in civil engineering.

If required, attach a page with the project engineer's seal with the following statement:

"I hereby state that this Construction Stormwater Pollution Prevention Plan for <u>(name of project)</u> has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and customary in this community for professional engineers. I understand that the City of Mercer Island does not and will not assume liability for the sufficiency, suitability, or performance of Construction SWPPP BMPs prepared by me."

Applicant Signature for Full Stormwater Package (Sections A through D)

I have read and completed the Stormwater Submittal Package and know the information provided to be true and correct.

Print Applicant Name:

Applicant Signature:

E. Cali___ Date

Table V-4.5.2(5) Maintenance Standards - Catch Basins Results					
Maintenance Component	Defect	Conditions When Maintenance is Needed	Expected When Main- tenance is performed		
General	Trash & Debris		No Trash or debris loc- ated imme- diately in front of catch basin or on grate open- ing. No trash or debris in the catch basin. Inlet and out- let pipes free of trash or debris. No dead animals or vegetation present within the catch basin.		
	Sediment	Sediment (in the basin) that exceeds 60 per- cent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of 6 inches clearance from the sediment surface to the invert of the lowest pipe.	No sediment in the catch basin		
	Frame and/or	Top slab has holes larger than 2 square inches or cracks wider than 1/4 inch. (Intent is to make sure no material is running into basin).	Top slab is free of holes and cracks. Frame is sit-		

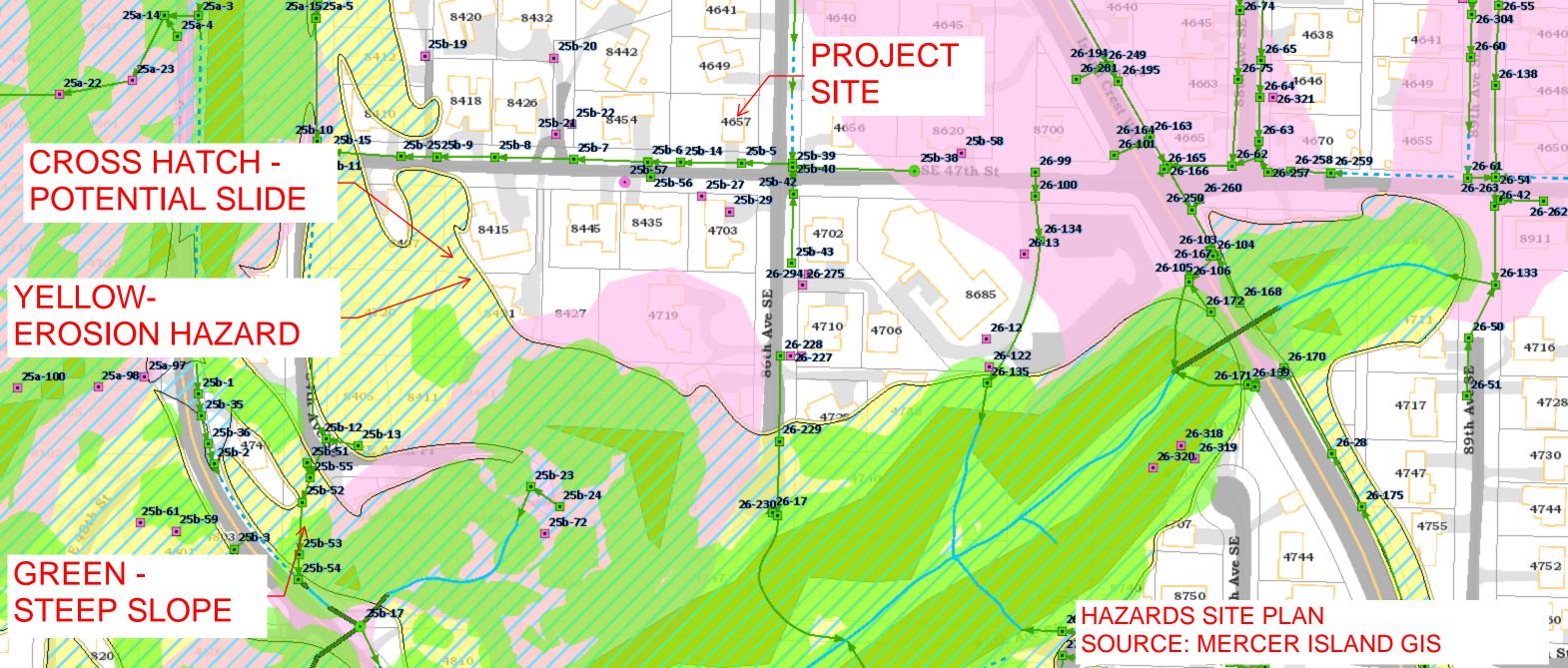
Table V-4.5.2(5) Maintenance Standards - Catch Basins

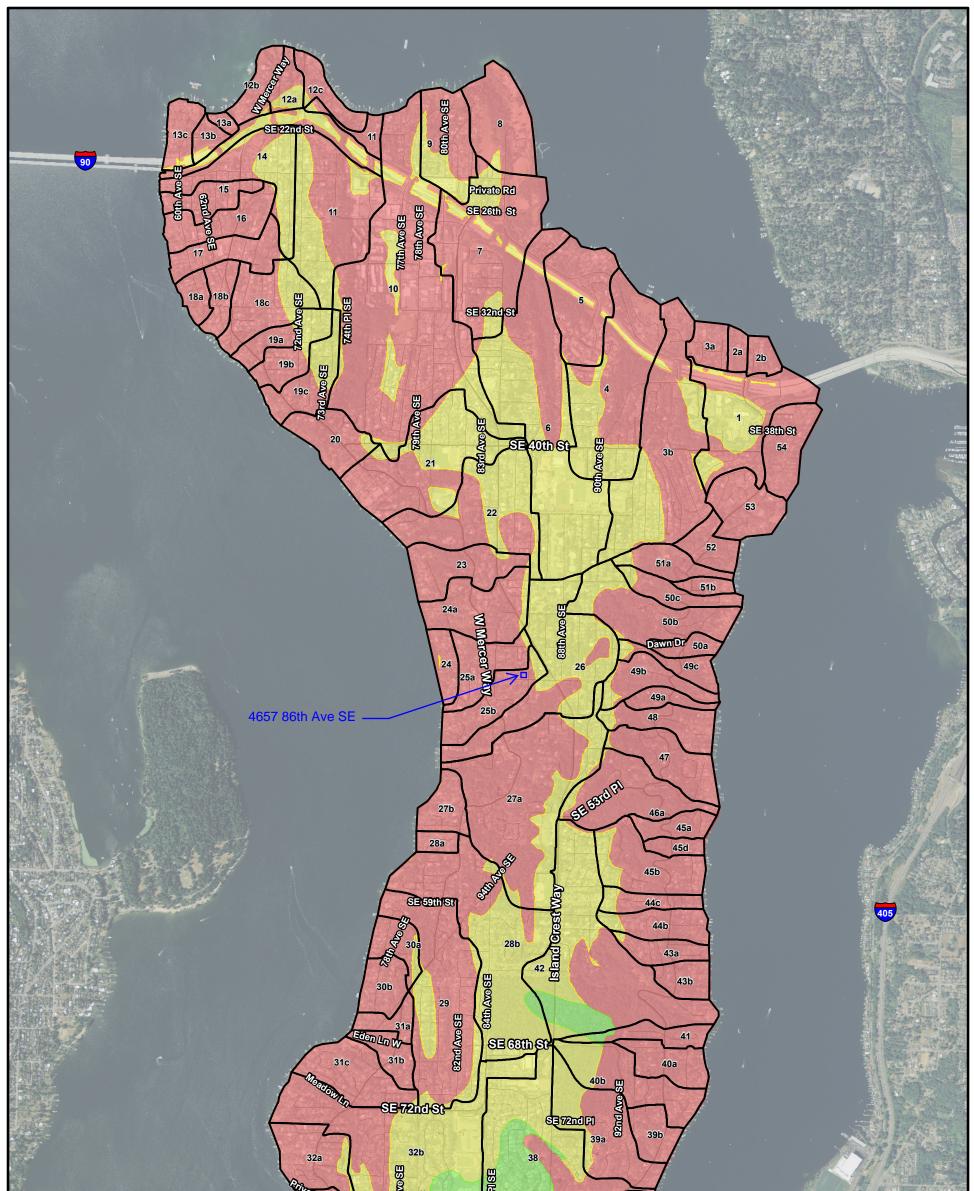
Maintenance Component	Detect	Conditions When Maintenance is Needed	Results Expected When Main- tenance is performed
		IF rame not citting tillen on ton clab I A con-	ting flush on the riser rings or top slab and firmly attached.
	Fractures or Cracks in	Maintenance person judges that structure is unsound. Grout fillet has separated or cracked wider than 1/2 inch and longer than 1 foot at the	Basin replaced or repaired to design stand- ards.
		joint of any inlet/outlet pipe or any evidence I of soil particles entering catch basin through r cracks.	regrouted and secure at basin wall.
			Basin replaced or repaired to design stand- ards.
	Vegetation	Vegetation growing across and blocking more than 10% of the basin opening.	No veget- ation block- ing opening to basin.
		Vegetation growing in inlet/outlet pipe joints that is more than six inches tall and less than six inches apart.	No veget- ation or root growth present.
	Contamination and Pollution	See "Detention Ponds" (No. 1)	No pollution present.
Catch Basin Cover	Cover Not in Place	Cover is missing or only partially in place. Any open catch basin requires main- tenance.	Catch basin cover is closed
	•	1 5	Mechanism opens with

Table V-4.5.2(5) Maintenance Standards - Catch Basins (continued)

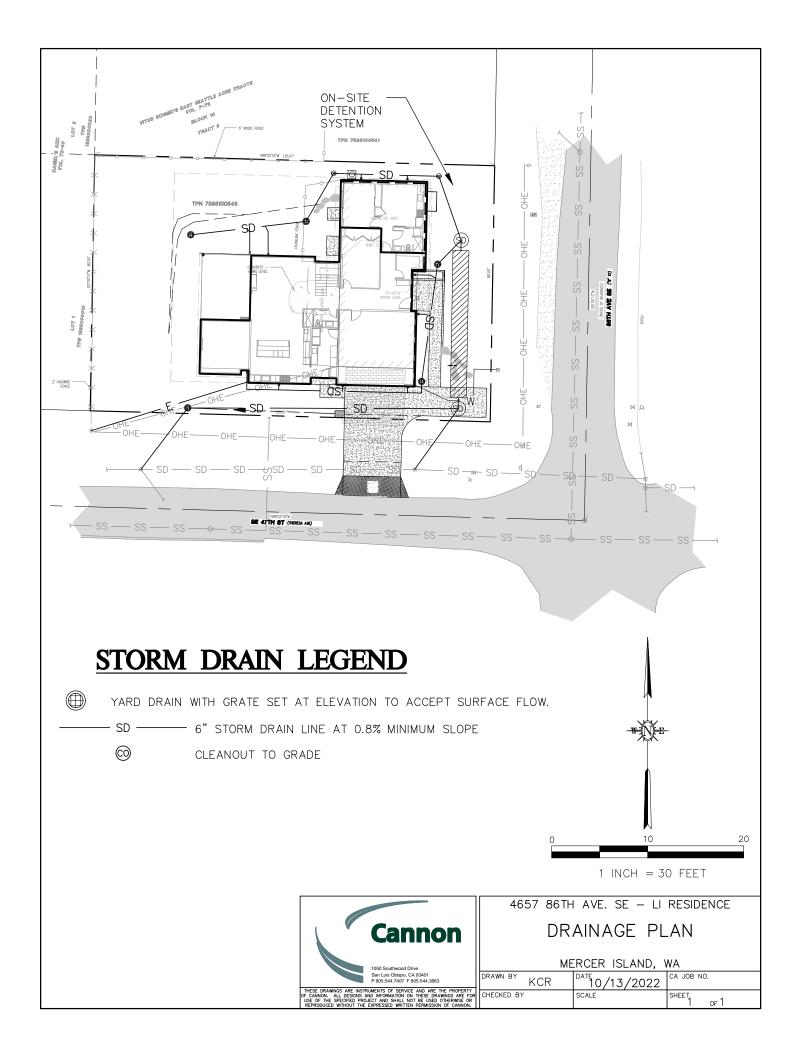
Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Main- tenance is performed
	Working	frame have less than 1/2 inch of thread.	proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. (Intent is keep cover from sealing off access	Cover can be removed by one main- tenance per-
		to maintenance.)	son.
Ladder	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs,not securely attached to basin wall, mis- alignment, rust, cracks, or sharp edges.	Ladder meets design stand- ards and allows main- tenance per- son safe access.
	Grate opening Unsafe	Grate with opening wider than 7/8 inch.	Grate open- ing meets design stand- ards.
Metal Grates (If Applic- able)	Trash and Debris	Trash and debris that is blocking morethan 20% of grate surface inletting capacity.	Grate free of trash and debris.
	Damaged or Missing.	Grate missing or broken member(s) of the grate.	Grate is in place and meets design standards.

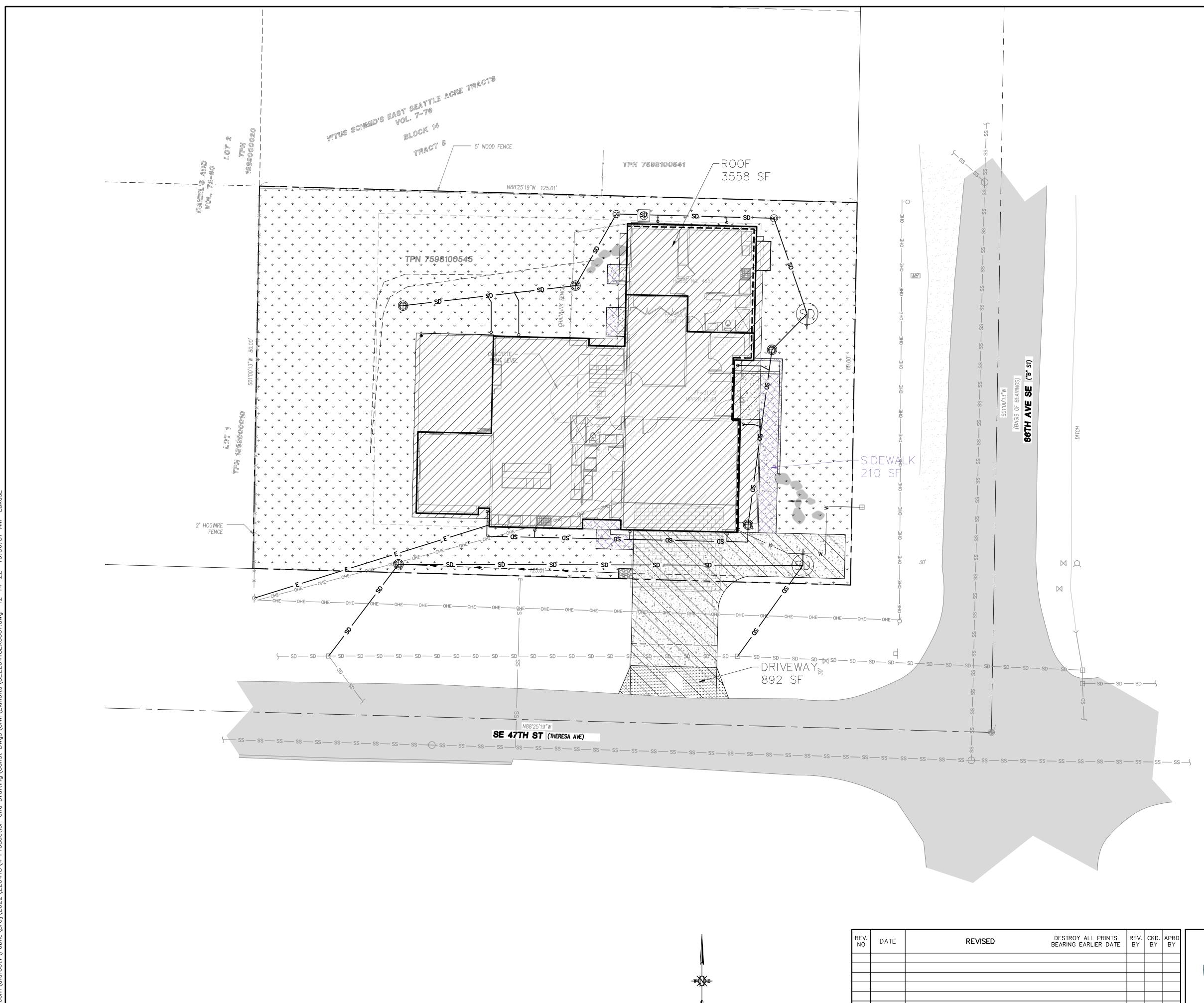
	Specific I	Table 3.2.4 Maintenance Requirements for Detention Vault	s/Tanks
Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Storage Area	Plugged Air Vents	One-half of the cross section of a vent is blocked at any point or the vent is damaged.	Vents open and functioning.
	Debris and Sediment	Accumulated sediment depth exceeds 10% of the diameter of the storage area for 1/2 length of storage vault or any point depth exceeds 15% of diameter.	All sediment and debris removed from storage area.
		(Example: 72-inch storage tank would require cleaning when sediment reaches depth of 7 inches for more than 1/2 length of tank.)	
	Joints Between Tank/Pipe Section	Any openings or voids allowing material to be transported into facility. (Will require engineering analysis to determine structural stability).	All joint between tank/pipe sections are sealed.
	Tank Pipe Bent Out of Shape	Any part of tank/pipe is bent out of shape more than 10% of its design shape. (Review required by engineer to determine structural stability).	Tank/pipe repaired or replaced to design.
	Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab	Cracks wider than 1/2-inch and any evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determines that the vault is not structurally sound.	Vault replaced or repaired to design specifications and is structurally sound.
		Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or any evidence of soil particles entering the vault through the walls.	No cracks more than 1/4- inch wide at the joint of the inlet/outlet pipe.
Manhole	Cover Not in Place	Cover is missing or only partially in place. Any open manhole requires maintenance.	Manhole is closed.
	Locking Mechanis m Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread (may not apply to self- locking lids).	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. Intent is to keep cover from sealing off access to maintenance.	Cover can be removed and reinstalled by one maintenance person.
	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, misalignment, not securely attached to structure wall, rust, or cracks.	Ladder meets design standards. Allows maintenance person safe access.





State Real and State	
Legend Infiltrating LID facilities may be feasible, 36 Storm drainage basin	Figure 3. Low impact development infiltration feasibility on Mercer Island.
and soil has high infiltration potential	N
Infiltrating LID facilities may be feasible, and soil has moderate infiltration potential	$\mathbf{\widehat{O}}$
Infiltrating LID facilities are not permitted	0 950 1,900 3,800 Feet
* Map is intended to be used for planning purposes only. Site-specific analysis is required prior to design and construction of LID facilities.	Aerial photography: USDA (2009) K1Projectsi10-04816-000ProjectNid_feasibility-report-11x17.mxd





	REV. DATE	REVISED	DESTROY ALL PRINTS REV. CKD. APR BEARING EARLIER DATE BY BY BY				LI RESIDENCE	-
-₩- ₩- 					Cannon		IENDMENT S ER ISLAND, WASH	
					1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863	DRAWN BY	DATE 10/11/2022	CA JOB NO. 220418
1 INCH = 10 FEET				CANNON. ALL DESIGNS OF THE SPECIFIED	NSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF AND INFORMATION ON THESE DRAWINGS ARE FOR USE PROJECT AND SHALL NOT BE USED OTHERWISE OR T THE EXPRESSED WRITTEN PERMISSION OF CANNON.	CHECKED BY KR	SCALE AS SHOWN	SHEET 1 OF 1

STORM DRAIN LEGEND

YARD DRAIN WITH GRATE SET AT ELEVATION TO ACCEPT SURFACE FLOW.

CLEANOUT TO GRADE

IMPERVIOU	S AREA (SF)
ROOF	3558
DRIVEWAY	892
SIDEWALK	210
LANDSCAPE	5802

GENERAL SITE NOTES

- 1. EXISTING FEATURES, TOPOGRAPHIC AND BOUNDARY INFORMATION SHOW ON THESE PLANS ARE FROM TOPOGRAPHIC SURVEY PROVIDED BY APEX, DATED 12/17/2021
- 2. ALL WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE FOLLOWING:
 - A. STANDARDS OF THE UNITED STATES DEPARTMENT OF LABOR, OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION, OFFICE OF STANDARDS AND RULES OF THE STATE DIVISION OF OCCUPATIONAL SAFETY AND HEALTH,
- B. THE REQUIREMENTS OF ALL PERMITS ISSUED FOR WORK BY THE CITY OF MERCER ISLAND. WHERE CONFLICTS EXIST BETWEEN ANY OF THE ABOVE LISTED SPECIFICATIONS, THE MOST STRINGENT LISTED SPECIFICATION SHALL APPLY.
- 3. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO SECURE ALL PERMITS NECESSARY TO PERFORM WORK. INCLUDING BUT NOT LIMITED TO WORK WITHIN THE PUBLIC RIGHT-OF-WAY, GRADING, TREE REMOVAL, AND UTILITY MODIFICATIONS.
- 4. CONTRACTOR SHALL SUPPLY ALL EQUIPMENT, LABOR, AND MATERIALS NECESSARY TO PERFORM THE WORK SHOWN ON THE APPROVED PLANS. 5. IT SHALL BE THE RESPONSIBILITY OF THE VARIOUS CONTRACTORS TO COORDINATE THEIR
- WORK SO AS TO ELIMINATE CONFLICTS AND WORK TOWARD THE GENERAL GOOD AND COMPLETION OF THE ENTIRE PROJECT.
- 6. ALL WORKMANSHIP AND MATERIALS FURNISHED BY THE CONTRACTOR SHALL BE OF GOOD QUALITY AND NEW. NEITHER FINAL ACCEPTANCE NOR FINAL PAYMENT BY THE OWNER SHALL RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR FAULTY MATERIALS OR WORKMANSHIP
- 7. IN THE EVENT OF ANY CONFLICT OF INFORMATION SHOWN ON THE APPROVED PLANS OR ANY CONFLICT BETWEEN THE APPROVED PLANS AND THE INTENT OF A CONSISTENT AND FUNCTIONAL PRODUCT, THE CONTRACTOR SHALL SO NOTIFY THE OWNER IN WRITING, UPON WHICH NOTICE THE OWNER SHALL RESOLVE THE CONFLICTS BY THE ISSUANCE OF A WRITTEN ORDER, REVISED PLANS, OR BOTH. THE CONTRACTOR SHALL BEAR FULL COST AND RESPONSIBILITY FOR WORK AFFECTED BY SUCH CONFLICTS AND PERFORMED BY CONTRACTOR PRIOR TO SUCH NOTICE TO THE OWNER AND ISSUANCE OF SUCH ORDER AND/OR REVISED PLANS.
- 8. CONTRACTOR SHALL EXERCISE ALL NECESSARY CAUTION TO AVOID DAMAGE TO ANY EXISTING TREES. OR SURFACE IMPROVEMENTS, OR TO ANY EXISTING DRAINAGE STRUCTURE, WATER STRUCTURE, SEWER CLEANOUTS, MANHOLES, OR JUNCTION BOXES FOR UNDERGROUND ELECTRIC, GAS, TELEPHONE, CABLE TV. STORM, SANITARY, WATER OR OTHER UTILITIES WHICH ARE TO REMAIN IN PLACE AND SHALL BEAR FULL RESPONSIBILITY FOR ANY DAMAGE THERETO.
- 9. ALL KNOWN EXISTING UTILITY LINES ARE SHOWN FOR INFORMATION ONLY. CONTRACTOR SHALL EXERCISE ALL NECESSARY CAUTION TO ANY EXISTING UTILITY LINES OR FACILITIES TO REMAIN IN PLACE. WHETHER OR NOT SUCH FACILITATES APPEAR ON THE APPROVED PLANS, AND SHALL BEAR FULL RESPONSIBILITY FOR ANY DAMAGE THERETO.
- 10. CONTRACTOR SHALL CONTACT THE "CALL BEFORE YOU DIG" UNDERGROUND UTILITY LOCATING SERVICE (811) AND THE AFFECTED UTILITY COMPANY PRIOR TO STARTING WORK TO REQUEST AND OBTAIN MARKING OF EXISTING UNDERGROUND FACILITIES.
- 11. CONTRACTORS SHALL HIRE A LICENSED LAND SURVEYOR TO PROVIDE CONSTRUCTION STAKING IN ORDER TO ENSURE THE PROJECT IS CONSTRUCTED TO THE LINES AND GRADES INDICATED ON THE APPROVED PLANS.
- 12. INSPECTION OF WORK: CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF REQUIRE INSPECTIONS WITH THE APPROPRIATE AGENCIES AND UTILITY COMPANIES AND CITY OF MEDINA STANDARDS.
- 13. ENGINEER OF RECORD SHALL BEAR NO RESPONSIBILITY FOR METHODS AND PROCEDURES OF WORK ESTABLISHED BY CONTRACTOR. JOBSITE CONDITIONS, JOBSITE SAFETY, OR CONFORMANCE WITH SAFETY PROCEDURES AND REQUIREMENTS.
- 14. IN CONFORMANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR THE JOBSITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL INDEMNIFY AND HOLD HARMLESS BOTH THE OWNER AND ENGINEER FROM ANY AND ALL LIABILITY REAL OR ALLEGED IN CONNECTION WITH THE PERFORMANCE OF THE WORK ON THIS PROJECT, EXCEPTING LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER OF RECORD.
- 15. ALL UNSUITABLE MATERIALS SHALL BE REMOVED FROM THE PROJECT AND BE PLACED AT A SUITABLE DISPOSAL SITE.
- 16. AN EMERGENCY ON-SITE BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP SYSTEM
- 17. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM.
- 18. THE LAWN AND LANDSCAPE AREAS ARE REQUIRED TO PROVIDE POST-CONSTRUCTION SOIL QUALITY AND DEPTH IN ACCORDANCE WITH BMP T5.13. THE PROJECT CIVIL ENGINEER MUST PROVIDE A LETTER OF CERTIFICATION TO ENSURE THAT THE LAWN AND LANDSCAPE AREAS ARE MEETING THE POST-CONSTRUCTION SOIL QUALITY AND DEPTH REQUIREMENTS SPECIFIED ON THE APPROVED PLAN SET PRIOR TO FINAL INSPECTION OF THE PROJECT.
- 19. AN EMERGENCY ON-SITE, BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP SYSTEM

20. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM.

PROJECT INFORMATION:

SITE ADDRESS: 4657 86TH AVE. SE, MERCER ISLAND, WA 98040 PARCEL NUMBER: 7598100545 LEGAL DESCRIPTION: (PER STATUTORY WARRANTY DEED, KING CO, REC. NO. 20211005001771)

THE SOUTH 80 FEET OF THE EAST 125 FEET OF TRACT 5 IN BLOCK 14 OF THE VITUS SCHMID'S EAST SEATTLE, ACRE TRACTS, AS PER PLAT RECORDED IN VOLUME 7 OF PLATS, PAGE 76, RECORDS OF KING COUNTY AUDITOR;

KING, STATE OF WASHINGTON.

UTILITIES:

WATER: CITY OF MERCER ISLAND 611 SE 36TH STREET, MERCER ISLAND, WA 98040 (206) 275-7602

<u>SEWER:</u> CITY OF MERCER ISLAND 611 SE 36TH STREET, MERCER ISLAND. WA 98040 (206) 275–7602

<u>FIRE:</u> MERCER ISLAND FIRE DEPARTMENT <u>ELECTRIC/GAS:</u> PSE

OWNER: PAUL LI

(703)965-9722

DEVELOPER: CAMERON WEAVER WW SUSTAINABLE, LLC 23613 140TH AVE. SE

KENT, WA 98042

<u>CIVIL ENGINEER:</u> CANNON

ATTN: KATIE ROLLINS PE: ED COLLINS PHONE: (425) 677-2325 1700 NW GILMAN BLVD, SUITE 100 ISSAQUAH, WA 98027

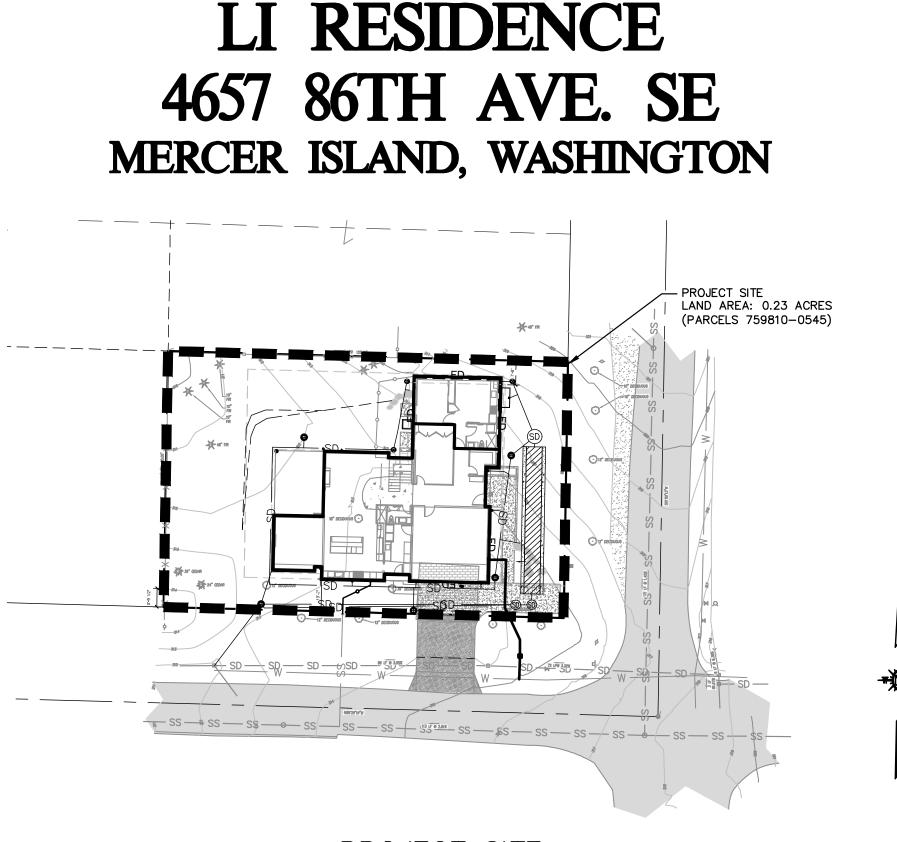
SURVEYOR:

APEX ENGINEERING LLC 2601 SOUTH 35TH STREET, SUITE 200 TACOMA, WA 98409 253 473-0599

ARCHITECT:

LURE. DESIGN SOLUTIONS 13842 NE 8TH STREET, #E102 BELLEVUE, WA 98005 (425) 870–0383

EXISTING UNDERGROUND UTILITY INFORMATION SHOWN ON THESE PLANS IS BASED UPON A COMBINATION OF SOURCES INCLUDING FIELD TOPOGRAPHIC SURVEY AND CITY SYSTEM MAPS. CONTRACTOR SHALL BE ADVISED THAT UNDERGROUND UTILITIES NOT IDENTIFIED ON THESE PLANS MAY EXIST WITHIN THE PROJECT SITE, AND SHALL EXERCISE APPROPRIATE CARE DURING EXCAVATION ACTIVITIES. CONTRACTOR SHALL POTHOLE AND FIELD VERIFY EXISTING UNDERGROUND UTILITY SIZE AND LOCATIONS AT CRITICAL LOCATIONS PRIOR TO CONSTRUCTION, AND NOTIFY ENGINEER IF DISCREPANCIES EXIST.



PROJECT SITE 1" = 30'

SITUATE IN THE CITY OF MERCER ISLAND, COUNTY OF

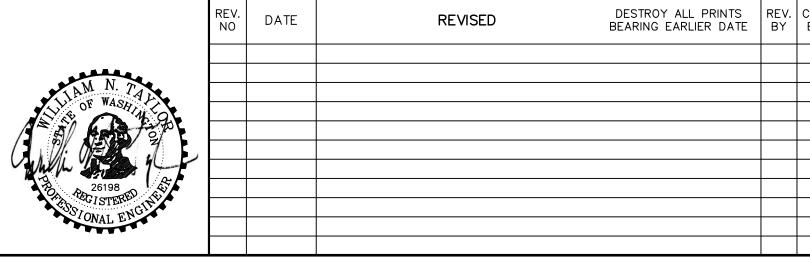


SHEET INDEX

No.	SHEET TITLE
C1	TITLE SHEET
C2	DRAINAGE & BMP PL
С3	UTILITY CONEECTIONS
C4	GRADING AND DRAIN
C5	GRADING AND DRAIN
C6	TESC PLAN
C7	TESC NOTES AND ST
C8	TESC DETAILS
C9	TESC DETAILS

VICINITY MAP

NTS



ABBREVIATIONS

DOWNSPOUT SPLASH

ASPHALT CONCRETE	AC	EDGE OF PAVEMENT	EP
CENTERLINE	CL / @	MANHOLE RIM ELEVATION	RIM
BACK OF WALK	BOW	MATCH EXISTING	ME
EACH WAY	E.W.	ON CENTER	0.C.
EXISTING GRADE	EG	RIGHT OF WAY	ROW
EXISTING	(E)	STANDARD	STD
FINISHED SURFACE	FS	STORM DRAIN	SD
FINISHED GRADE	FG	SANITARY SEWER	SS
HIGH POINT	HP	TOP OF CURB	TC
INVERT	INV	TOP OF GRATE	TG
MINIMUM	MIN	BOTTOM OF WALL	BW
PROPERTY LINE	PL / PL	TOP OF WALL	TW
PUBLIC UTILITY EASEMENT	PUE	UNLESS NOTED OTHERWISE	U.N.O.
		UNIFORM PLUMBING CODE	UPC
LEGEND			

<u>EXISTING</u>

<u>PROPOSED</u>

LOT BOUNDARY PROPERTY LINE EASEMENT/SETBACK STREET CENTERLINE BUILDING		
GRADE BREAK CONTOURS	= = = =	
WATER MAIN OR LATERAL SANITARY SEWER STORM DRAIN LINE GAS LINE FOUNDATION DRAIN	SS	W SS SD FD RD
DRY UTILITY LATERAL		KD JT
AC PAVING CONCRETE		
STORM DRAIN INLET		
CLEANOUT TO GRADE		69
YARD DRAIN		
JUNCTION BOX		\bigcirc
CATCH BASIN		

IS PLAN AGE DETAILS AGE DETAILS

ANDARD DETAILS



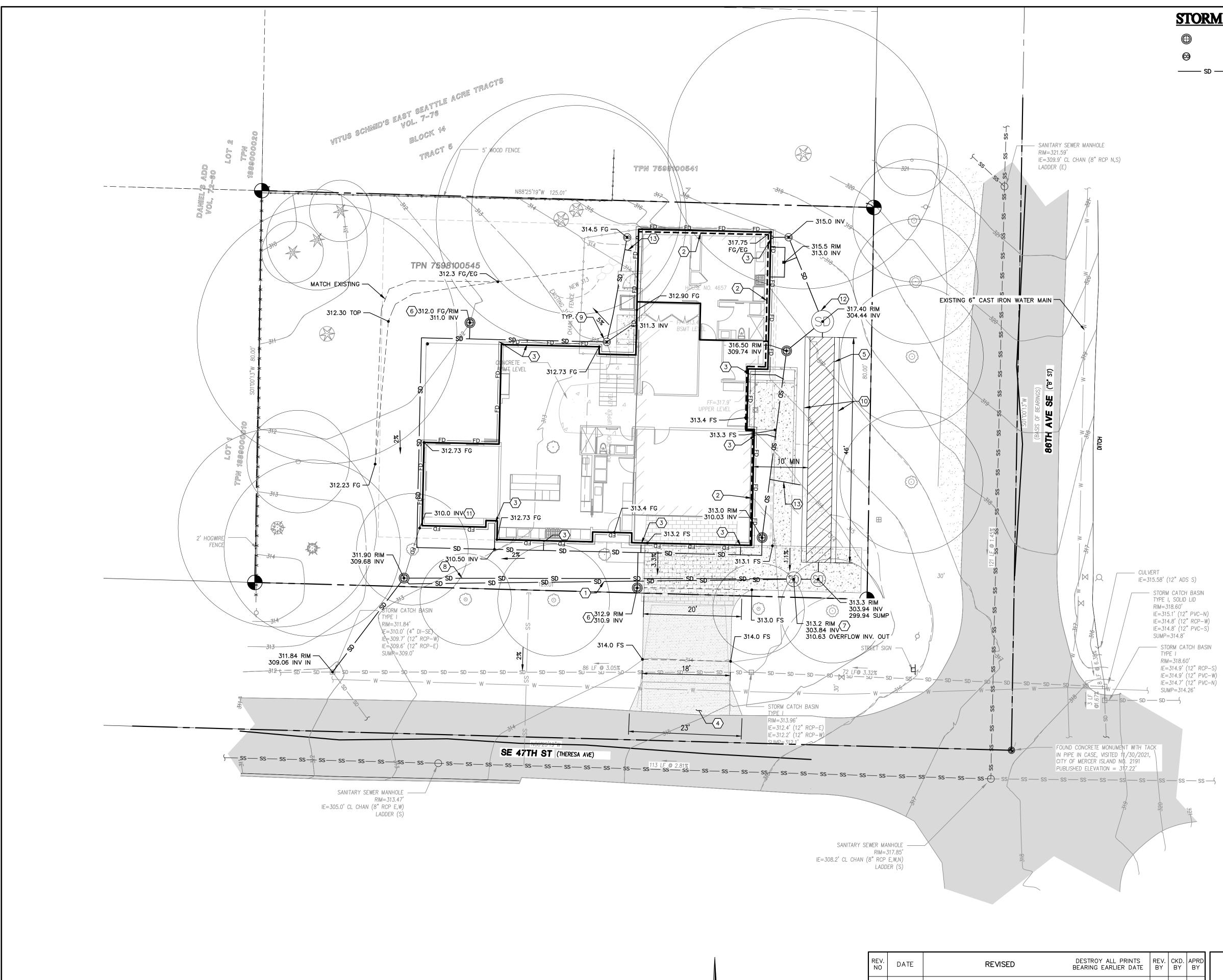
LI RESIDENCE

TITLE SHEET

MERCER ISLAND, WASHINGTON

CKD. BY	APRD BY	
		Cannon
		1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863
		THESE DRAWINGS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF CANNON. ALL DESIGNS AND INFORMATION ON THESE DRAWINGS ARE FOR USE OF THE SPECIFIED PROJECT AND SHALL NOT BE USED OTHERWISE OR REPRODUCED WITHOUT THE EXPRESSED WRITTEN PERMISSION OF CANNON.

DRAWN BY	DATE	CA JOB NO.		
SEM	3/14/2023	220418		
CHECKED BY	SCALE	SHEET		
KR	AS SHOWN	C1 OF 8		



		REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	CKD BY
- #	LILOF WASH						
	While the get						
10	20 26198 PEGISTERED IN						
1 INCH = 10 FEET	ONAL D						

STORM DRAIN LEGEND

- YARD DRAIN WITH GRATE SET AT ELEVATION TO ACCEPT SURFACE FLOW. CLEANOUT TO GRADE
- ------ SD ------ 6" STORM DRIAN LINE AT 0.8% MINIMUM SLOPE

CONSTRUCTION NOTES

- $\langle 1 \rangle$ 6" OVERFLOW DISCHARGE PIPE.
- 2 EXISTING RAISED FOOTING TO REMAIN. REUSE EXISTING FOUNDATION DRAIN WHERE POSSIBLE (SEE STORM DRAIN NOTE 3).
- $\langle 3 \rangle$ CONNECT DOWNSPOUT STORMDRAIN LINE.
- ASPHALT RURAL DRIVEWAY APRON PER MODIFIED KING COUNTY
- 5 60" DIA. DETENTION PIPE, PER DETAIL A SHEET C5. GEOTECHNICAL ENGINEER TO PROVIDE MEASURES TO PROTECT EXISTING FOUNDATION DURING EXCAVATION.
- $\langle 6 \rangle$ Nyloplast catch basin, per detail a sheet C5 (grated cover). DUAL PUMP OUTLET STRUCTURE, PER DETAIL 1 SHEET C5. THE PUMP
- STATION STRUCTURE SHALL BE A TYPE 2 CB, WITH DIAMETER SIZED TO HOUSE THE SELECTED PUMPS AND RELATED EQUIPMENT WITH ADEQUATE ROOM FOR MAINTENANCE ACCESS (MINIMUM 48 DIA.), OR MANUFACTURER'S PACKAGED PUMP STATION STRUCTURE SUBJECT TO REVIEW AND APPROVAL FROM THE PROJECT ENGINEER
- $\langle 8 \rangle$ EMERGENCY OVERFLOW PIPE.
- $\langle 9 \rangle$ STORM DRAIN CLEANOUT (SOLID COVER).
- (10) CONCRETE PIPE ANCHORS PER DETAIL 2 ON SHEET C5. GEOTECHNICAL ENGINEER TO DETERMINE IF GROUNDWATER IS PRESENT AT BOTTOM OF EXCAVATION AND IF ANCHORS ARE REQUIRED.
- $\langle 11 \rangle$ FOOTING DRAIN CONNECTION POINT TO TIGHTLINE.
- (12) UPPER CATCH BASIN PER DETAIL A SHEET C5
- $\langle 13 \rangle$ provide back of wall drains behind all rockeries

STORM DRAINAGE NOTES

- 1. STORM PIPE SHALL BE PVC CONFORMING TO ASTM D-3034 SDR 35 (4" - 15") OR ASTM F-679 (18"-27"). BEDDING AND BACKFILL SHALL BE AS SHOWN IN THE STANDARD DETAILS.
- 2. INSTALL FOOTING DRAINS AROUND ALL BUILDING PERIMETER FOOTINGS. THE FOOTING DRAINAGE SYSTEM AND THE ROOF DOWNSPOUT SYSTEM SHALL NOT BE INTERCONNECTED AND SHALL SEPARATELY CONVEY COLLECTED FLOWS TO THE CONVEYANCE SYSTEM OR TO ON-SITE STORMWATER FACILITIES.
- 3. EXISTING FOOTING DRAIN MUST BE TV INSPECTED. IF THE RESULT OF THE TV INSPECTION IS NOT IN SATISFACTORY CONDITION AS DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING FOOTING DRAIN IS REQUIRED.
- 4. CONNECT ALL ROOF DRAIN DOWNSPOUTS TO BELOW GROUND STORM DRAIN SYSTEM, UNLESS SPLASH BLOCK SHOWN.
- 5. YARD DRAIN CATCH BASINS SHALL BE NYLOPLAST DRAIN BASIN WITH 8 CIRCLE GRATE (UNLESS OTHERWISE NOTED) OR APPROVED EQUAL (DRAIN BASIN SHALL BE 12" MIN OR LARGER TO ACCOMMODATE CONNECTED PIPES). SEE DETAIL C ON SHEET C4. CATCH BASINS WITHIN DRIVEWAY OR OTHER VEHICULAR AREAS SHALL HAVE TRAFFIC RATED GRATE AND FRAME. SEE NOTE 8.
- 6. PIPES ROUTED BELOW RETAINING WALLS SHALL BE SUFFICIENTLY PROTECTED FROM WALL LOADING. FOR LANDSCAPE WALLS, PROVIDE 6" SLURRY ENCASEMENT OF PIPE AT MINIMUM WITHIN THE ZONE OF INFLUENCE OF WALL.
- 7. YARD DRAIN TYPE AND MANUFACTURER FOR YARD DRAINS THAT ARE NOT ON THE MAIN STORM DRAIN CONVEYANCE LINE SHALL BE COORDINATED BY LANDSCAPE ARCHITECT.
- 8. IF THE EXISTING CATCH BASIN IS NOT IN SATISFACTORY CONDITION, AS DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING CATCH BASIN IS REQUIRED.
- 9. WHERE TWO STORM DRAINS INTERSECT AND THERE IS NO PROPOSED INLET, PROVIDE CONNECTION WITH WYE/T JUNCTION

GENERAL NOTES

- 1. ALL GRADING AND DRAINAGE SHALL CONFORM TO THE CURRENT BUILDING CODE.
- 2. ALL DISTURBED PERVIOUS AREA SHALL BE AMENDED PER BMP T5.13 POST-CONSTRUCTION SOIL QUALITY AND DEPTH (SEE DETAIL B, SHEET C4)
- 3. AN EMERGENCY ON-SITE, BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP SYSTEM
- 4. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM

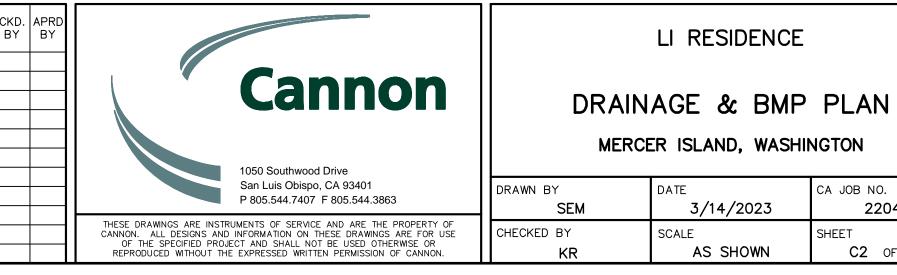
GRADING NOTES

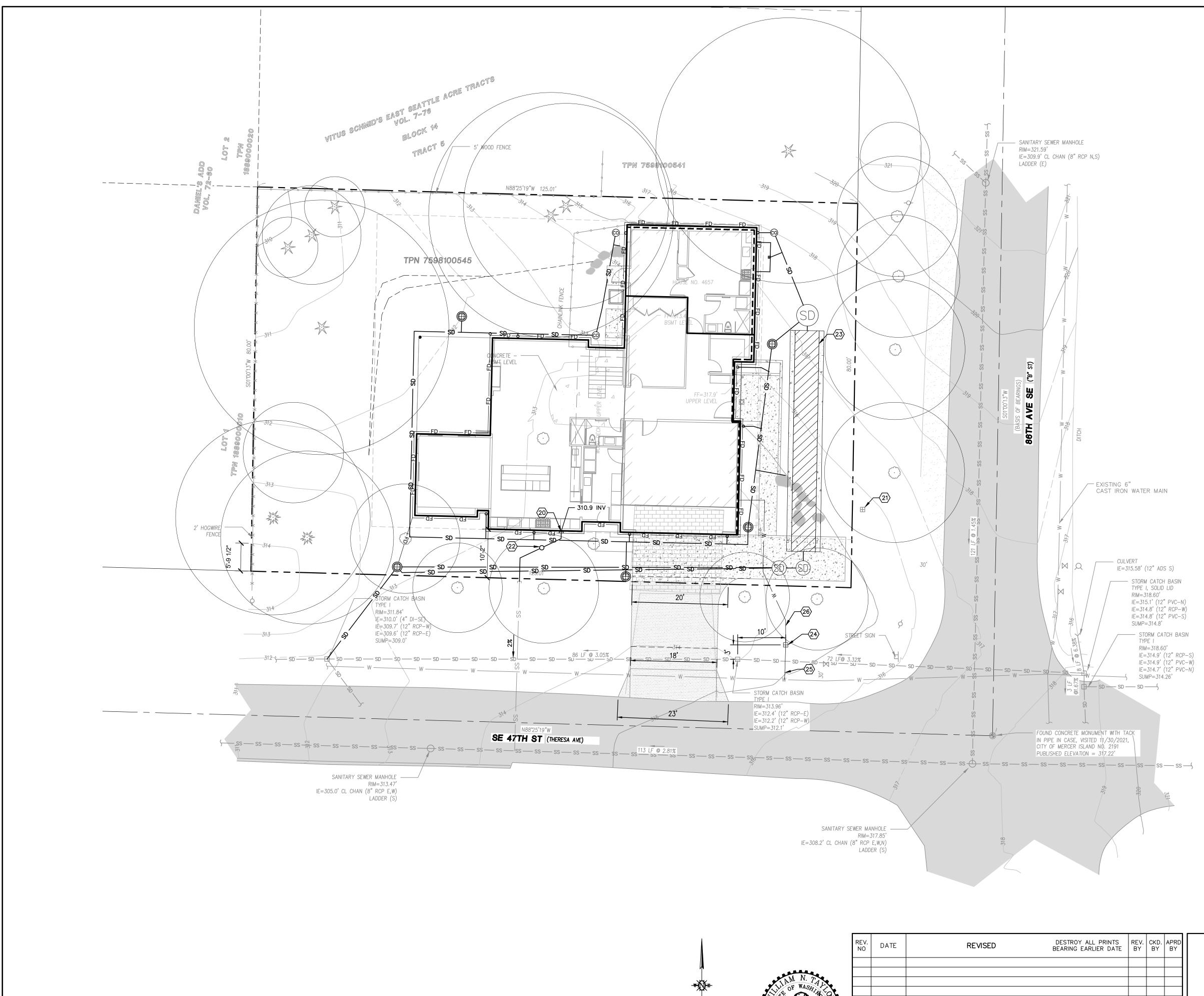
- 1. PRIOR TO CONSTRUCTING FLATWORK NEAR EXTERIOR DOORS, CONTRACTOR SHALL COORDINATE DOOR THRESHOLD DROP WITH ARCHITECTURAL PLANS.
- 2. ALL FLATWORK ADJACENT TO BUILDINGS SHALL BE SLOPED AT A MINIMUM OF 2% FOR 10' AWAY FROM THE BUILDING UNLESS NOTED OTHERWISE.
- 3. FINISHED GRADE (PERVIOUS AREA) DIRECTLY ADJACENT TO BUILDINGS SHALL BE SLOPED AT 5% MINIMUM FOR 10' AWAY FROM THE BUILDING, OR TO A DESIGNATED SWALE SLOPED AT 2% MINIMUM.
- 4. FINISHED GRADE (DIRT OR LANDSCAPE AREA) DIRECTLY OUTSIDE OF THE BUILDING SHALL BE 8" MINIMUM AND 12" MAXIMUM BELOW FINISHED FLOOR UNLESS NOTED WITH A SPECIALLY DESIGNED FOOTING. FOOTING EMBEDMENT SHOULD MEET MINIMUM REQUIREMENTS PER STRUCTURAL ENGINEER.
- 5. NO SLOPES SHALL EXCEED 2:1 HORIZONTAL TO VERTICAL.
- 6. DRIVEWAYS SHALL NOT EXCEED 20% MAX SLOPE THROUGH EXPECTED DRIVE PATH.
- 7. CONTRACTOR TO INSTALL STAIRS, STAIR LANDINGS AND HANDRAILS AS REQUIRED BY THE WASHINGTON BUILDING CODE.

220418

C2 OF 8

8. PRIOR TO CONSTRUCTION, PROJECT GEOTECHNICAL ENGINEER SHALL REVIEW AND APPROVE THESE PLANS FOR CONFORMANCE WITH THEIR





0	10	20
	1 INCH = 10 FEET	

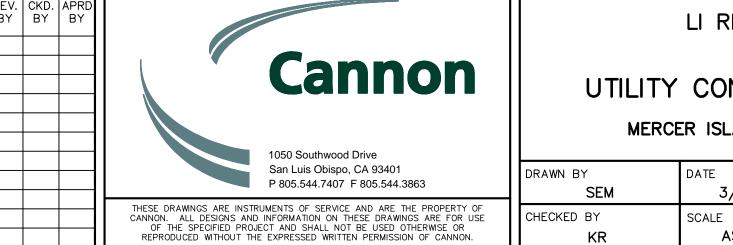
1111

CONSTRUCTION NOTES

- CONNECT PROPOSED SEWER TO EXISTING MAIN USING 6" SDR 35 PVC. PER CITY OF MERCER ISLAND SEWER STD DETAIL S-18 & S-22 ON SHEET C4
- $\fbox{21}$ Existing water meter box to be removed and existing water service to be abandoned at the main.
- 22 SEWER CHECK VALVE PER CITY OF MERCER ISLAND SEWER STD DETAIL S-18
- $\langle 23 \rangle$ on-site detention system.
- 24 CONSTRUCT NEW 1" WATER METER PER STANDARD W-13
- $\langle 25 \rangle$ construct 1" water service line per standard w-13
- (26) CONSTRUCT 1.25" WATER SUPPLY LINE TO BUILDING

UTILITY NOTES

- 1. THE TV INSPECTION OF THE EXISTING SIDE SEWER TO THE CITY SEWER MAIN ON SE 47TH ST IS REQUIRED PRIOR TO ANY WORK RELATED TO THE SIDE SEWER. IF THE RESULT OF THE TV INSPECTION IS NOT IN SATISFACTORY CONDITION, AS DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING SIDE SEWER IS REQUIRED.
- 2. FRANCHISE UTILITIES ARE NOT REVIEWED OR APPROVED BY THE CITY OF MERCER ISLAND,

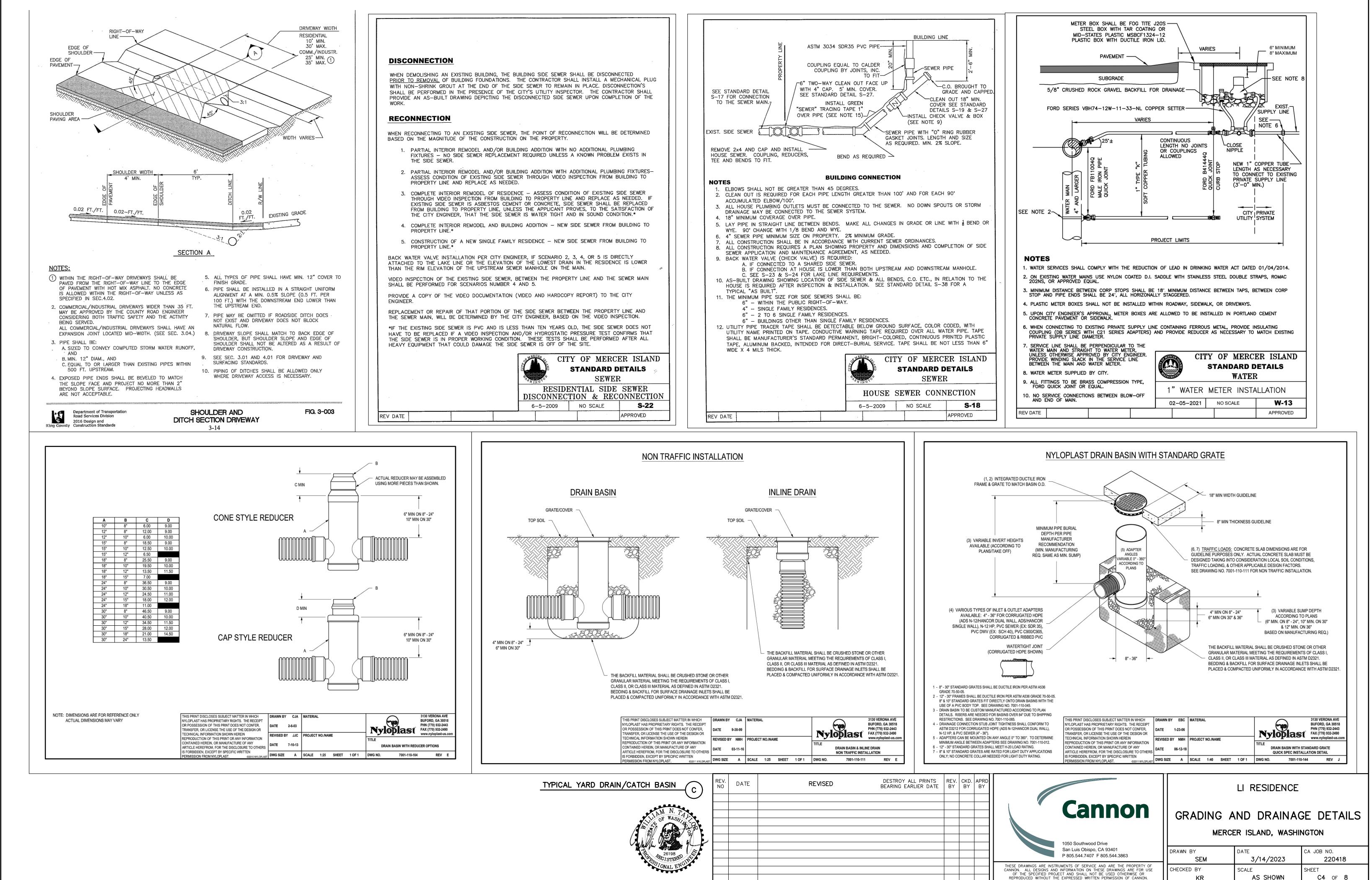


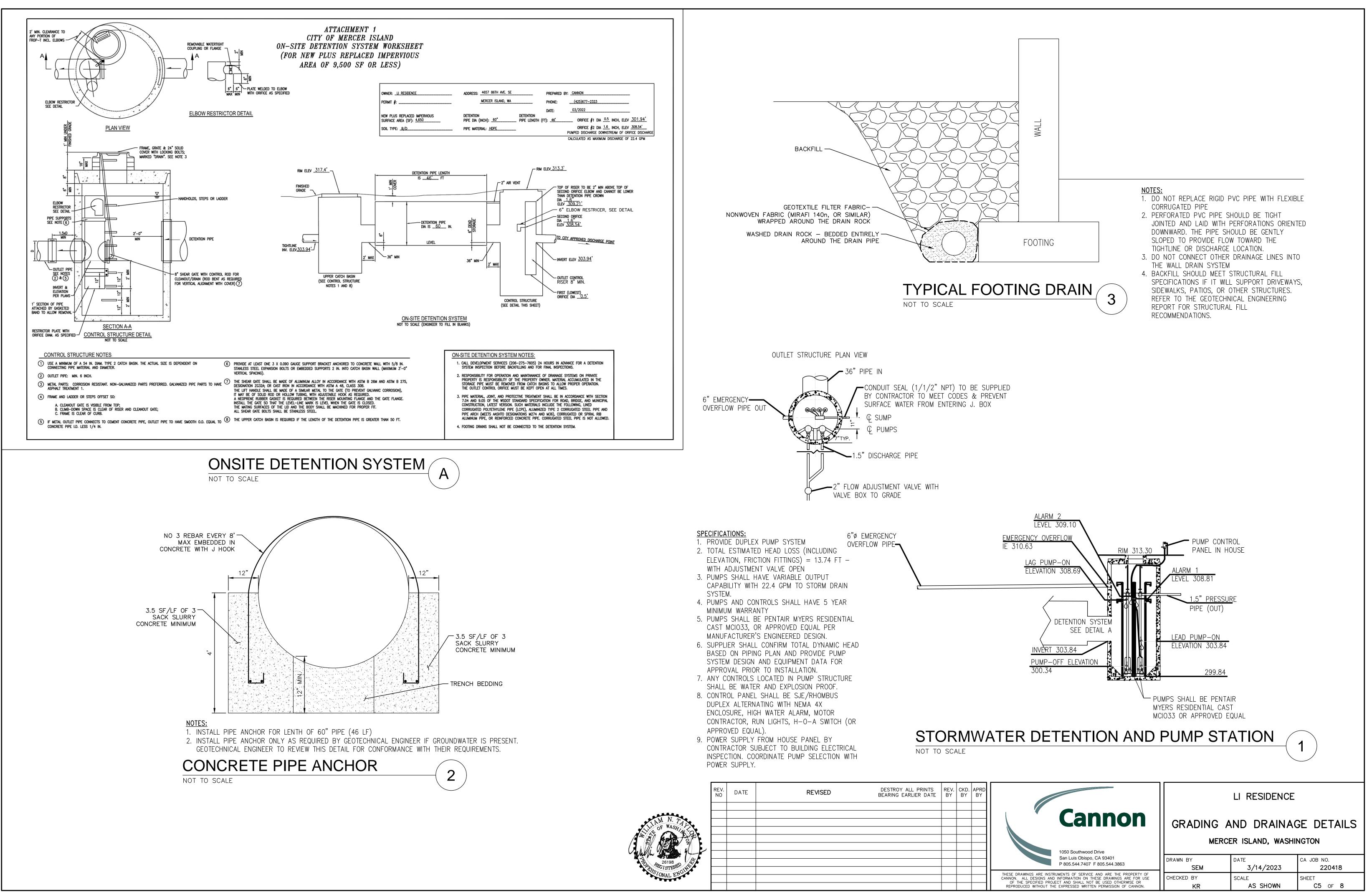
LI RESIDENCE

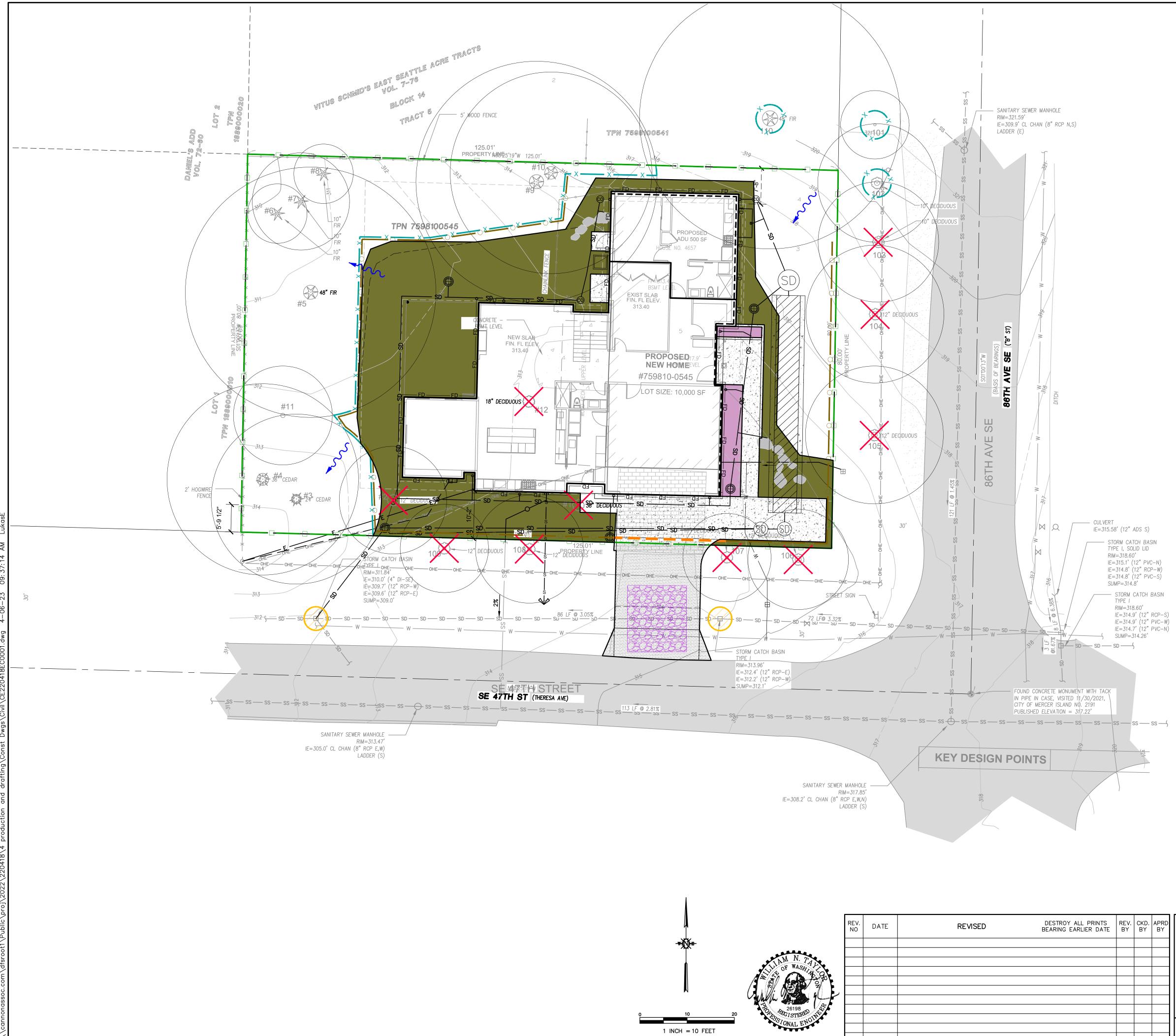
UTILITY CONECTIONS PLAN

MERCER ISLAND, WASHINGTON

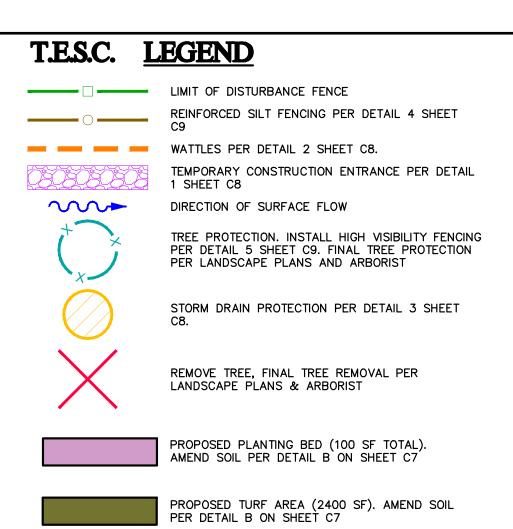
DATE	CA JOB NO.
3/14/2023	220418
SCALE	SHEET
AS SHOWN	C3 OF 8
	3/14/2023 SCALE







-#	REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. CKD. APRD BY BY BY			LI RESIDENCE	
THIAM N. TAL						1050 Southwood Drive	MERC	TESC PLAN CER ISLAND, WASH	
0 10 20						San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863	DRAWN BY SEM	DATE 3/14/2023	CA JOB NO. 220418
1 INCH = 10 FEET						THESE DRAWINGS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF CANNON. ALL DESIGNS AND INFORMATION ON THESE DRAWINGS ARE FOR USE OF THE SPECIFIED PROJECT AND SHALL NOT BE USED OTHERWISE OR REPRODUCED WITHOUT THE EXPRESSED WRITTEN PERMISSION OF CANNON.	CHECKED BY KR	SCALE AS SHOWN	SHEET C6 OF 8



T.E.S.C. NOTES

1. ALL DISTURBED SOIL AREAS SHALL BE TREATED WITH SOIL AMENDMENT PER DETAIL B ON SHEET C4.

2. CONTRACTOR TO INSTALL AND MAINTAIN ALL EROSION CONTROL BMP'S AS SHOWN ON THIS PLAN IN ACCORDANCE WITH DEPARTMENT OF ECOLOGY STORMWATER POLLUTION PREVENTION REQUIREMENTS SUMMARIZED ON SHEETS C7 & C9. IF BMP INSTALLATION AND MAINTENANCE IN INADEQUATE TO MEET THESE REQUIREMENTS CONTRACTOR TO PROVIDE ADDITIONAL BMPS AT NO ADDITIONAL COST TO OWNER.

CONFIRM TREES TO BE PROTECTED, REMOVED, AND PRESERVATION MEASURES WITH PROJECT ARBORISTS.

DISCHARGE CLEAN, NON-TURBID DEWATERING WATER DIRECTLY TO SURFACE WATER. DEWATERING WATER WHICH HAS SIMILAR CHARACTERISTICS TO STORMWATER RUNOFF AT THE SITE SHOULD BE DISCHARGED TO SEDIMENT TRAP

3. ALL CONSTRUCTION FENCING TO NOT EXCEED 3' HEIGHT WITHIN RIGHT OF WAY.

IE=314.9' (12" RCP-S) IE=314.9' (12" PVC-Ŵ) IE=314.7' (12" PVC-N)

ESC STANDARD NOTES

- 1. APPROVAL OF THIS EROSION/SEDIMENTATION CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAINAGE DESIGN (E.G. SIZE AND LOCATION OF ROADS, PIPES, RESTRICTORS, CHANNELS, RETENTION FACILITIES, UTILITIES).
- MAINTENANCE, REPLACE-MENT, AND UPGRADING OF THESE ESC FACILITIES IS THE RESPONSIBILITY OF THE APPLIC-ANT/CONTRACTOR UNTIL ALL CONSTRUCTION IS COMPLETED AND APPROVED AND VEGETATION/LANDSCAPING IS ESTABLISHED.
- SHALL BE CLEARLY FLAGGED IN THE FIELD PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION PERIOD, NO DISTURBANCE BEYOND THE FLAGGED CLEARING LIMITS SHALL BE PERMITTED. THE FLAGGING SHALL BE MAINTAINED BY THE APPLIC-ANT/CONTRACTOR FOR THE DURATION OF CONSTRUCTION.
- 4. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN CONJUNCTION WITH ALL CLEAR-ING AND GRADING ACTIVITIES, AND IN SUCH A MANNER AS TO INSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT ENTER THE DRAINAGE SYSTEM, ROADWAYS, OR VIOLATE APPLICABLE WATER STANDARDS.
- 5. THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD, THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT LEAVE THE SITE.
- 6. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/CONTRACTOR AND MAINTAINED AS NECESSARY TO ENSURE THEIR CONTINUED FUNCTIONING.
- 7. THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH OR WITHIN THE 48 HOURS FOLLOWING A MAJOR STORM EVENT.
- 8. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A TRAPPED CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAV-ING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT LADEN WATER INTO THE DOWNSTREAM SYSTEM.
- 9. STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES MAY BE REQUIRED TO INSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT.

BMP T5.13: Post-Construction Soil Quality and Depth

Applications and Limitations

Establishing a minimum soil quality and depth is not the same as preservation of naturally occurring soil and vegetation. However, establishing a minimum soil quality and depth will provide improved on-site management of stormwater flow and water quality.

Soil organic matter can be attained through numerous materials such as compost, composted woody material, biosolids, and forest product residuals. It is important that the materials used to meet the soil quality and depth BMP be appropriate and beneficial to the plant cover to be established. Likewise, it is important that imported topsoils improve soil conditions and do not have an excessive percent of clay fines.

This BMP can be considered infeasible on till soil slopes greater than 33 percent.

Design Guidelines

- Soil retention. Retain, in an undisturbed state, the duff layer and native topsoil to the maximum extent practicable. In any areas requiring grading remove and stockpile the duff layer and topsoil on site in a designated, controlled area, not adjacent to public resources and critical areas, to be reapplied to other portions of the site where feasible.
- Soil quality. All areas subject to clearing and grading that have not been covered by impervious surface, incorporated into a drainage facility or engineered as structural fill or slope shall, at project completion, demonstrate the following:
- 1. A topsoil layer with a minimum organic matter content of 10% dry weight in planting beds, and 5% organic matter content in turf areas, and a pH from 6.0

to 8.0 or matching the pH of the undisturbed soil. The topsoil layer shall have a minimum depth of eight inches except where tree roots limit the depth of incorporation of amendments needed to meet the criteria. Subsoils below the topsoil layer should be scarified at least 4 inches with some incorporation of the upper material to avoid stratified layers, where feasible.

2. Mulch planting beds with 2 inches of organic material

Total Quan	ds: 100 SF x 9.3 CY ÷ 1,000 SF= 1 CY	Product:
------------	--------------------------------------	----------

2. THE IMPLEMENTATION OF THESE ESC PLANS AND THE CONSTRUCTION,

3. THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN

CONSTRUCTION SEQUENCE

1. POST SIGN WITH NAME AND PHONE NUMBER OF ESC SUPERVISOR.

HOLD THE PRE-CONSTRUCTION MEETING.

- 2. FENCE CLEARING LIMITS.
- 3. INSTALL CATCH BASIN PROTECTION.
- 4. GRADE AND INSTALL CONSTRUCTION ENTRANCES.
- 5. INSTALL PERIMETER PROTECTION (SILT FENCE, BRUSH BARRIER, ETC.).
- 6. CONSTRUCT SURFACE WATER CONTROLS SIMULTANEOUSLY WITH CLEARING AND GRADING FOR PROJECT DEVELOPMENT.
- 7. INSTALL STORM DRAINAGE AND SANITARY SEWER SYSTEMS.
- 8. INSTALL WATER SYSTEM, IRRIGATION, AND DRY UTILITIES AS SHOWN ON
- 9. INSTALL ASPHALT CONCRETE PAVEMENT, CURBING AND CEMENT CONCRETE AS SHOWN ON PLANS.
- 10. MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH CITY OF MERCER ISLAND STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.
- 11. RELOCATE EROSION CONTROL MEASURES, OR INSTALL NEW MEASURES SO THAT AS SITE CONDITIONS CHANGE, THE EROSION AND SEDIMENT CONTROL IS ALWAYS IN ACCORDANCE WITH THE CITY OF MERCER ISLAND CLEARING AND GRADING STANDARDS.
- 12. COVER ALL AREAS THAT WILL BE UNWORKED FOR MORE THAN SEVEN DAYS DURING THE WET SEASON WITH STRAW, WOOD FIBER MULCH, COMPOST, PLASTIC SHEETING, OR EQUIVALENT.
- 13. STABILIZED ALL AREAS WITHIN SEVEN DAYS OF REACHING FINAL GRADE. 14. SEED, SOD, STABILIZE, OR COVER ANY AREAS TO REMAIN UNWORKED
- FOR MORE THAN 30 DAYS. 15. UPON COMPLETION OF THE PROJECT, STABILIZED ALL DISTURBED AREAS AND REMOVE BMPS IF APPROPRIATE.

3.	Use compost and other materials that meet these organic content require-
	ments:

- a. The organic content for "pre-approved" amendment rates can be met only using compost meeting the compost specification for BMP T7.30: Bioretention Cells, Swales, and Planter Boxes (p.959), with the exception that the compost may have up to 35% biosolids or manure. The compost must also have an organic matter content of 40% to 65%,
- and a carbon to nitrogen ratio below 25:1. The carbon to nitrogen ratio may be as high as 35:1 for plantings composed entirely of plants native to the Puget Sound Lowlands region.
- b. Calculated amendment rates may be met through use of composted material meeting (a.) above; or other organic materials amended to meet the carbon to nitrogen ratio requirements, and not exceeding the contaminant limits identified in Table 220-B, Testing Parameters, in WAC 173-350-220.

The resulting soil should be conducive to the type of vegetation to be established.

- Implementation Options: The soil quality design guidelines listed above can be met by using one of the methods listed below:
- 1. Leave undisturbed native vegetation and soil, and protect from compaction during construction.
- 2. Amend existing site topsoil or subsoil either at default "pre-approved" rates, or at custom calculated rates based on tests of the soil and amendment.
- 3. Stockpile existing topsoil during grading, and replace it prior to planting. Stockpiled topsoil must also be amended if needed to meet the organic matter or depth requirements, either at a default "pre-approved" rate or at a custom calculated rate.
- 4. Import topsoil mix of sufficient organic content and depth to meet the requirements.

More than one method may be used on different portions of the same site. Soil that already meets the depth and organic matter quality standards, and is not compacted, does not need to be amended.

SOIL QUALITY AND DEPTH

	REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. CKD. APRD BY BY BY				LI RESIDENCE	Ξ
							Cannon		ES AND STAND rcer island, wash	
R AN						S	950 Southwood Drive an Luis Obispo, CA 93401 805.544.7407 F 805.544.3863	DRAWN BY SEM	DATE 3/14/2023	CA JOB NO. 220418
10.						CANNON. ALL DESIGNS AND INF OF THE SPECIFIED PROJECT	NTS OF SERVICE AND ARE THE PROPERTY OF ORMATION ON THESE DRAWINGS ARE FOR USE AND SHALL NOT BE USED OTHERWISE OR PRESSED WRITTEN PERMISSION OF CANNON.	CHECKED BY	SCALE AS SHOWN	SHEET C7 OF 8

	GUIDE TO MULCH MATERIALS, RATES AND USE									
MULCH MATERIAL	QUALITY STANDARDS	APPLICATION RATES	REMARKS							
STRAW	AIR DRIED; FREE FROM UNDESIRABLE SEED & COARSE MATERIAL.	2"-3" THICK; 5 BALES PER 1000SF OR 2-3 TONS PER ACRE	COST-EFFECTIVE PROTECTION WHEN APPLIED WITH ADEQUATE THICKNESS. HAND-APPLICATION GENERALLY REQUIRES GREATER THICKNESS THAN BLOWN STRAW. THE THICKNESS OF STRAW MAY BE REDUCED BY HALF WHEN USED IN CONJUNCTION WITH SEEDING. IN WINDY AREAS STRAW MUST BE HELD IN PLACE BY CRIMPING, USING A TACKIFIER, OR COVERING WITH NETTING. BLOWN STRAW ALWAYS HAS TO BE HELD IN PLACE WITH A TACKIFIER AS EVEN LIGHT WINDS WILL BLOW IT AWAY. STRAW, HOWEVER, HAS SEVERAL DEFICIENCIES THAT SHOULD BE CONSIDERED WHEN SELECTING MULCH MATERIALS. IT OFTEN INTRODUCES AND/OR ENCOURAGES THE PROPAGATION OF WEED SPECIES AND IT HAS NO SIGNIFICANT LONG-TERM BENEFITS. STRAW SHOULD BE USED ONLY IF MULCHES WITH LONG-TERM BENEFITS ARE UNAVAILABLE LOCALLY. IT SHOULD ALSO NOT BE USED WITHIN THE ORDINARY HIGH-WATER ELEVATION OF SURFACE WATERS (DUE TO FLOTATION).							
HYDROMULCH	NO GROWTH INHIBITING FACTORS.	APPOX. 25–30 LBS PER 1000 SF OR 1500–2000 LBS PER ACRE	SHALL BE APPLIED WITH HYDROMULCHER. SHALL NOT BE USED WITHOUT SEED AND TACKIFIER UNLESS THE APPLICATION RATE IS AT LEAST DOUBLED. FIBERS LONGER THAN ABOUT ¾-1 INCH CLOG HYDROMULCH EQUIPMENT. FIBERS SHOULD BE KEPT TO LESS THAN ¾ INCH.							
COMPOSTED MULCH AND COMPOST	NO VISIBLE WATER OR DUST DURING HANDLING. MUST BE PURCHASED FROM SUPPLIER WITH SOLID WASTE HANDLING PERMIT (UNLESS EXEMPT).	2" THICK MIN.; APPROX. 100 TONS PER ACRE (APPROX. 800 LBS PER YARD)	MORE EFFECTIVE CONTROL CAN BE OBTAINED BY INCREASING THICKNESS TO 3". EXCELLENT MULCH FOR PROTECTING FINAL GRADES UNTIL LANDSCAPING BECAUSE IT CAN BE DIRECTLY SEEDED OR TILLED INTO SOIL AS AN AMENDMENT. COMPOSTED MULCH HAS A COARSER SIZE GRADATION THAN COMPOST. IT IS MORE STABLE AND PRACTICAL TO USE IN WET AREAS AND DURING RAINY WEATHER CONDITIONS.							
CHIPPED SITE VEGETATION	AVERAGE SIZE SHALL BE SEVERAL INCHES. GRADATIONS FROM FINES TO 6 INCHES IN LENGTH FOR TEXTURE, VARIATION, AND INTERLOCKING PROPERTIES.	2" MINIMUM THICKNESS	THIS IS A COST-EFFECTIVE WAY TO DISPOSE OF DEBRIS FROM CLEARING AND GRUBBING, AND IT ELIMINATES THE PROBLEMS ASSOCIATED WITH BURNING. GENERALLY, IT SHOULD NOT BE USED ON SLOPES ABOVE APPROX. 10% BECAUSE OF ITS TENDENCY TO BE TRANSPORTED BY RUNOFF. IT IS NOT RECOMMENDED WITHIN 200 FEET OF SURFACE WATERS. IF SEEDING IS EXPECTED SHORTLY AFTER MULCH, THE DECOMPOSITION OF THE CHIPPED VEGETATION MAY TIE UP NUTRIENTS IMPORTANT TO GRASS ESTABLISHMENT.							
WOOD-BASED MULCH	NO VISIBLE WATER OR DUST DURING HANDLING. MUST BE PURCHASED FROM A SUPPLIER WITH A SOLID WASTE HANDLING PERMIT OR ONE EXEMPT FROM SOLID WASTE REGULATIONS.	2" THICK; APPROX. 100 TONS PER ACRE (APPROX. 800 LBS. PER CUBIC YARD)	THIS MATERIAL IS OFTEN CALLED "HOG OR HOGGED FUEL." IT IS USABLE AS A MATERIAL FOR STABILIZED CONSTRUCTION ENTRANCES (BMP C105) AND AS A MULCH. THE USE OF MULCH ULTIMATELY IMPROVES THE ORGANIC MATTER IN THE SOIL. SPECIAL CAUTION IS ADVISED REGARDING THE SOURCE AND COMPOSITION OF WOODBASED MULCHES. ITS PREPARATION TYPICALLY DOES NOT PROVIDE ANY WEED SEED CONTROL, SO EVIDENCE OF RESIDUAL VEGETATION IN ITS COMPOSITION OR KNOWN INCLUSION OF WEED PLANTS OR SEEDS SHOULD BE MONITORED AND PREVENTED (OR MINIMIZED).							

BMP C105: Stabilized Construction Entrance / Exit

Purpose

Stabilized Construction entrances are established to reduce the amount of sediment transported onto paved roads by vehicles or equipment. This is done by constructing a stabilized pad of quarry spalls at entrances and exits for construction sites.

Conditions of Use

Construction entrances shall be stabilized wherever traffic will be entering or leaving a construction site if paved roads or other paved areas are within 1,000 feet of the site.

For residential construction provide stabilized construction entrances for each residence, rather than only at the main subdivision entrance. Stabilized surfaces shall be of sufficient length/width to provide vehicle access/parking, based on lot size/configuration.

On large commercial, highway, and road projects, the designer should include enough extra materials in the contract to allow for additional stabilized entrances not shown in the initial Construction SWPPP. It is difficult to determine exactly where access to these projects will take place; additional materials will enable the contractor to install them where needed.

Design and Installation Specifications

See Figure II-4.1.1 Stabilized Construction Entrance (p.273) for details. Note: the 100' minimum length of the entrance shall be reduced to the maximum practicable size when the size or configuration of the site does not allow the full length (100').

Construct stabilized construction entrances with a 12-inch thick pad of 4-inch to 8-inch quarry spalls, a 4-inch course of asphalt treated base (ATB), or use existing pavement. Do not use crushed concrete, cement, or calcium chloride for construction entrance stabilization because these products raise pH levels in stormwater and concrete discharge to surface waters of the State is prohibited.

A separation geotextile shall be placed under the spalls to prevent fine sediment from pumping up into the rock pad. The geotextile shall meet the following standards:

Grab Tensile Strength (ASTM D4751)	200 psi min.
Grab Tensile Elongation (ASTM D4632)	30% max.
Mullen Burst Strength (ASTM D3786-80a)	400 psi min.
AOS (ASTM D4751)	20-45 (U.S. standard sieve size)

- Consider early installation of the first lift of asphalt in areas that will paved; this can be used as a stabilized entrance. Also consider the installation of excess concrete as a stabilized entrance. During large concrete pours, excess concrete is often available for this purpose.
- Fencing (see BMP C103: High Visibility Fence (p.269)) shall be installed as necessary to restrict traffic to the construction entrance.
- Whenever possible, the entrance shall be constructed on a firm, compacted subgrade. This can substantially increase the effectiveness of the pad and reduce the need for maintenance.
- Construction entrances should avoid crossing existing sidewalks and back of walk drains if at all possible. If a construction entrance must cross a sidewalk or back of walk drain, the full length of the sidewalk and back of walk drain must be covered and protected from sediment leaving the site.

Maintenance Standards

Quarry spalls shall be added if the pad is no longer in accordance with the specifications.

- If the entrance is not preventing sediment from being tracked onto pavement, then alternative measures to keep the streets free of sediment shall be used. This may include replacement/cleaning of the existing quarry spalls, street sweeping, an increase in the dimensions of the entrance, or the installation of a wheel wash.
- Any sediment that is tracked onto pavement shall be removed by shoveling or street sweeping. The sediment collected by sweeping shall be removed or stabilized on site. The pavement shall not be cleaned by washing down the street, except when high efficiency sweeping is ineffective and there is a threat to public safety. If it is necessary to wash the streets, the construction of a small sump to contain the wash water shall be considered. The sediment would then be washed into the sump where it can be controlled.
- Perform street sweeping by hand or with a high efficiency sweeper. Do not use a non-high efficiency mechanical sweeper because this creates dust and throws soils into storm systems or conveyance ditches.
- Any quarry spalls that are loosened from the pad, which end up on the roadway shall be removed immediately.
- If vehicles are entering or exiting the site at points other than the construction entrance(s), fencing (see BMP C103) shall be installed to control traffic.
- Upon project completion and site stabilization, all construction accesses intended as permanent access for maintenance shall be permanently stabilized.

BMP C235: Wattles

Purpose

Wattles are temporary erosion and sediment control barriers consisting of straw, compost, or other material that is spread the flow of rill and sheet runoff, and can capture and retain sediment.

Conditions of Use

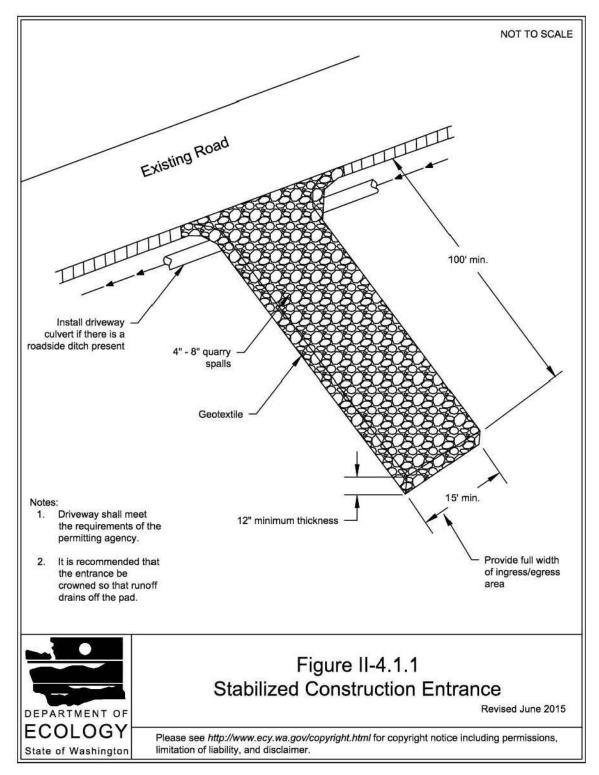
- · Use wattles:

- two seasons.
- between them.

Design Criteria

- See Figure II-3.24: Wattles for typical construction details.
- Install wattles perpendicular to the flow direction and parallel to the slope contour.

Figure II-4.1.1 Stabilized Construction Entrance



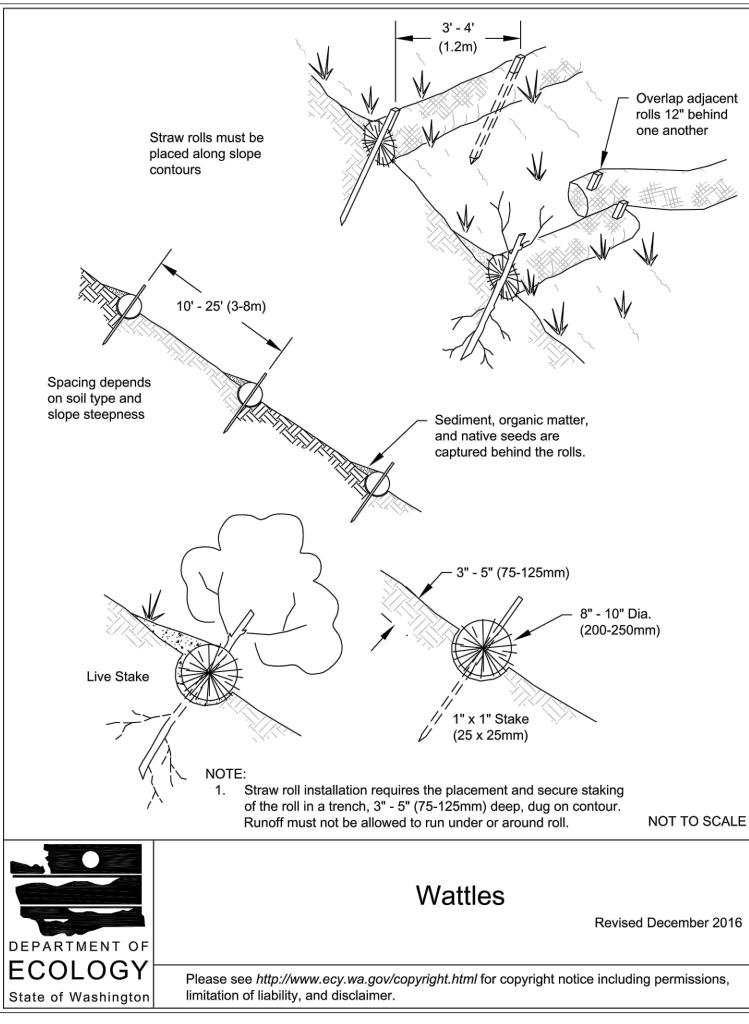
STABILIZED CONSTRUCTION ENTRANCE

- wrapped in netting made of natural plant fiber or similar encasing material. They reduce the velocity and can
- Wattles shall consist of cylinders of plant material such as weed-free straw, coir, wood chips, excelsior, or wood fiber or shavings encased within netting made of natural plant fibers unaltered by synthetic materials.
 - In disturbed areas that require immediate erosion protection.
 - On exposed soils during the period of short construction delays, or over winter months.
 - On slopes requiring stabilization until permanent vegetation can be established.
- The material used dictates the effectiveness period of the wattle. Generally, wattles are effective for one to
- Prevent rilling beneath wattles by entrenching and overlapping wattles to prevent water from passing
- Wattles are typically 8 to 10 inches in diameter and 25 to 30 feet in length.

 Place wattles in shallow trenches, staked along the contour of disturbed or newly constructed slopes. Dig narrow trenches across the slope (on contour) to a depth of 3- to 5-inches on clay soils and soils with gradual slopes. On loose soils, steep slopes, and areas with high rainfall, the trenches should be dug to a depth of 5- to 7- inches, or 1/2 to 2/3 of the thickness of the wattle.

-

• Start building trenches and installing wattles from the base of the slope and work up. Spread excavated material evenly along the uphill slope and compact it using hand tamping or other methods.



1/21/2021

- BMP C235: Wattles
- Construct trenches at intervals of 10- to 25-feet depending on the steepness of the slope, soil type, and rainfall. The steeper the slope the closer together the trenches.
- Install the wattles snugly into the trenches and overlap the ends of adjacent wattles 12 inches behind one another.
- Install stakes at each end of the wattle, and at 4-foot centers along entire length of wattle.
- If required, install pilot holes for the stakes using a straight bar to drive holes through the wattle and into the soil.
- Wooden stakes should be approximately 0.75 x 0.75 x 24 inches min. Willow cuttings or 3/8-inch rebar can also be used for stakes.
- Stakes should be driven through the middle of the wattle, leaving 2 to 3 inches of the stake protruding above the wattle.

Maintenance Standards

- Wattles may require maintenance to ensure they are in contact with soil and thoroughly entrenched, especially after significant rainfall on steep sandy soils.
- Inspect the slope after significant storms and repair any areas where wattles are not tightly abutted or water has scoured beneath the wattles.

WATTLES

	REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	CKD. AF BY B	PRD 3Y			LI RESIDENCE	
THIN N. TAL								Cannon	MER	TESC DETAIL	
26198 DEGISTERED NE								1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863	DRAWN BY SEM	DATE 3/14/2023	CA JOB NO. 220418
ONAL ENG								THESE DRAWINGS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF CANNON. ALL DESIGNS AND INFORMATION ON THESE DRAWINGS ARE FOR USE OF THE SPECIFIED PROJECT AND SHALL NOT BE USED OTHERWISE OR REPRODUCED WITHOUT THE EXPRESSED WRITTEN PERMISSION OF CANNON.	CHECKED BY KR	SCALE AS SHOWN	SHEET C8 OF 8

BMP C220: Storm Drain Inlet Protection

Purpose

Storm drain inlet protection prevents coarse sediment from entering drainage systems prior to permanent stabilization of the disturbed area.

Conditions of Use

Use storm drain inlet protection at inlets that are operational before permanent stabilization of the disturbed drainage area. Provide protection for all storm drain inlets downslope and within 500 feet of a disturbed or construction area, unless conveying runoff entering catch basins to a sediment pond or trap.

Also consider inlet protection for lawn and yard drains on new home construction. These small and numerous drains coupled with lack of gutters in new home construction can add significant amounts of sediment into the roof drain system. If possible delay installing lawn and yard drains until just before landscaping or cap these drains to prevent sediment from entering the system until completion of landscaping. Provide 18inches of sod around each finished lawn and yard drain.

Table II-4.2.2 Storm Drain Inlet Protection (p.358) lists several options for inlet protection. All of the methods for storm drain inlet protection tend to plug and require a high frequency of maintenance. Limit drainage areas to one acre or less. Possibly provide emergency overflows with additional end-of-pipe treatment where stormwater ponding would cause a hazard.

Type of Inlet Protection	Emergency Overflow	Applicable for Paved/ Earthen Surfaces	Conditions of Use
Drop Inlet Prote	ction		
Excavated drop inlet protection	Yes, tem- porary flood- ing will occur	Earthen	Applicable for heavy flows. Easy to maintain. Large area Require- ment: 30'x30'/acre
Block and gravel drop inlet protection	Yes	Paved or Earthen	Applicable for heavy concentrated flows. Will not pond.
Gravel and wire drop inlet pro- tection	No		Applicable for heavy concentrated flows. Will pond. Can withstand traffic.
Catch basin fil- ters	Yes	Paved or Earthen	Frequent Maintenance required.
Curb Inlet Prote	ction		л.
Curb inlet pro- tection with wooden weir	Small capacity overflow	Paved	Used for sturdy, more compact installation.
Block and gravel curb inlet protection	Yes	Paved	Sturdy, but limited filtration.
Culvert Inlet Pro	otection		
Culvert inlet Sed iment trap	-		18 month expected life.

Maintenance Standards

- Inspect catch basin filters frequently, especially after storm events. Clean and replace clogged inserts. For systems with clogged stone filters: pull away the stones from the inlet and clean or replace. An alternative approach would be to use the clogged stone as fill and put fresh stone around the inlet.
- Do not wash sediment into storm drains while cleaning. Spread all excavated material evenly over the surrounding land area or stockpile and stabilize as appropriate.

STORM DRAIN INLET PROTECTTION



BMP C233: Silt Fence

Purpose

Use of a silt fence reduces the transport of coarse sediment from a construction site by providing a temporary physical barrier to sediment and reducing the runoff velocities of overland flow. See Figure II-4.2.12 Silt Fence (p.369) for details on silt fence construction.

Conditions of Use

Silt fence may be used downslope of all disturbed areas.

- Silt fence shall prevent soil carried by runoff water from going beneath, through, or over the top of the silt fence, but shall allow the water to pass through the fence.
- Silt fence is not intended to treat concentrated flows, nor is it intended to treat substantial amounts of overland flow. Convey any concentrated flows through the drainage system to a sediment pond.
- Do not construct silt fences in streams or use in V-shaped ditches. Silt fences do not provide an adequate method of silt control for anything deeper than sheet or overland flow.

- Joints in filter fabric shall be spliced at posts. Use staples, wire rings or equivalent to attach fabric to posts 2"x2" by 14 Ga. wire or equivalent, if standard strength fabric used NINININININININININININININININI Minimum · 4"x4" trench Post spacing may be increased 2"x2" wood posts, steel to 8' if wire backing is used fence posts, or equivalent 2"x2" by 14 Ga. wire or equivalent, if standard strength fabric used Filter fabric -2' min Backfill trench with native soil or 3/4" -1.5" washed gravel Minimum -4"x4" trench fence posts, or equivalent NOT TO SCALE Figure II-4.2.12 Silt Fence Revised October 2014 DEPARTMENT O ECOLOGY Please see http://www.ecy.wa.gov/copyright.html for copyright notice including permissions, State of Washington limitation of liability, and disclaimer.

Design and Installation Specifications

• Do not allow flows greater than 0.5 cfs.

ards (p.370)):

. Use in combination with sediment basins or other BMPs.

Maximum slope steepness (normal (perpendicular) to fence line) 1H:1V.

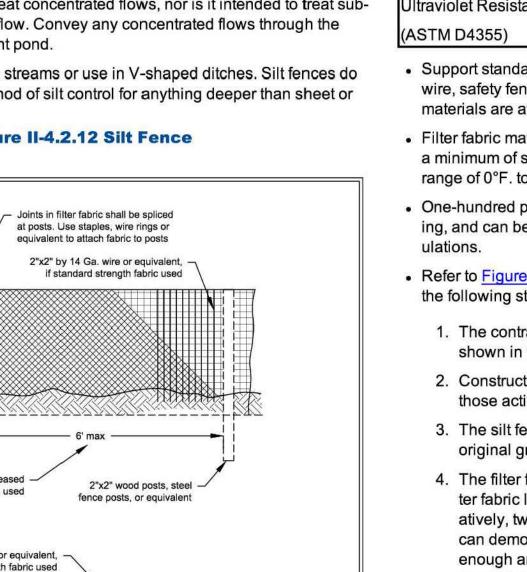
• The geotextile used shall meet the following standards. All geotextile properties lis-

ted below are minimum average roll values (i.e., the test result for any sampled roll

in a lot shall meet or exceed the values shown in Table II-4.2.3 Geotextile Stand-

Maximum sheet or overland flow path length to the fence of 100 feet.

Figure II-4.2.12 Silt Fence



- 5. Attach the filter fabric on the up-slope side of the posts and secure with tearing.
- 6. Support the filter fabric with wire or plastic mesh, dependent on the properties of the geotextile selected for use. If wire or plastic mesh is used, fasten the mesh securely to the up-slope side of the posts with the filter fabric up-slope of the mesh.
- 7. Mesh support, if used, shall consist of steel wire with a maximum mesh spacing of 2-inches, or a prefabricated polymeric mesh. The strength of the wire or polymeric mesh shall be equivalent to or greater than 180 lbs. grab tensile strength. The polymeric mesh must be as resistant to the same level of ultraviolet radiation as the filter fabric it supports.
- 8. Bury the bottom of the filter fabric 4-inches min. below the ground surface. Backfill and tamp soil in place over the buried portion of the filter fabric, so that no flow can pass beneath the fence and scouring cannot occur. When wire or polymeric back-up support mesh is used, the wire or polymeric mesh shall extend into the ground 3-inches min.
- 9. Drive or place the fence posts into the ground 18-inches min. A 12-inch min. depth is allowed if topsoil or other soft subgrade soil is not present and 18inches cannot be reached. Increase fence post min. depths by 6 inches if the fence is located on slopes of 3H:1V or steeper and the slope is perpendicular to the fence. If required post depths cannot be obtained, the posts shall be adequately secured by bracing or guying to prevent overturning of the fence due to sediment loading.
- 10. Use wood, steel or equivalent posts. The spacing of the support posts shall

REINFORCED SILT FENCE



Table II-4.2.3 Geotextile Standards

Polymeric Mesh AOS (ASTM D4751)	0.60 mm maximum for slit film 0.30 mm maximum for all othe 0.15 mm minimum for all fabric
Water Permittivity (ASTM D4491)	0.02 sec ⁻¹ minimum
Grab Tensile Strength	180 lbs. Minimum for extra stre
(ASTM D4632)	100 lbs minimum for standard
Grab Tensile Strength (ASTM D4632)	30% maximum
Ultraviolet Resistance (ASTM D4355)	70% minimum

 Support standard strength fabrics with wire mesh, chicken wire, 2-inch x 2-inch wire, safety fence, or jute mesh to increase the strength of the fabric. Silt fence materials are available that have synthetic mesh backing attached.

- Filter fabric material shall contain ultraviolet ray inhibitors and stabilizers to provide a minimum of six months of expected usable construction life at a temperature range of 0°F. to 120°F.
- One-hundred percent biodegradable silt fence is available that is strong, long lasting, and can be left in place after the project is completed, if permitted by local reg-
- Refer to Figure II-4.2.12 Silt Fence (p.369) for standard silt fence details. Include
- 1. The contractor shall install and maintain temporary silt fences at the locations shown in the Plans.
- 2. Construct silt fences in areas of clearing, grading, or drainage prior to starting those activities.
- 3. The silt fence shall have a 2-feet min. and a 21/2-feet max. height above the original ground surface.
- 4. The filter fabric shall be sewn together at the point of manufacture to form filter fabric lengths as required. Locate all sewn seams at support posts. Alternatively, two sections of silt fence can be overlapped, provided the Contractor can demonstrate, to the satisfaction of the Engineer, that the overlap is long enough and that the adjacent fence sections are close enough together to prevent silt laden water from escaping through the fence at the overlap.
- staples, wire, or in accordance with the manufacturer's recommendations Attach the filter fabric to the posts in a manner that reduces the potential for

woven (#30 sieve). er geotextile types (#50 sieve). c types (#100 sieve).

ngth fabric.	
strength fabric.	

the following standard Notes for silt fence on construction plans and specifications:

be a maximum of 6-feet. Posts shall consist of either:

- Wood with dimensions of 2-inches by 2-inches wide min. and a 3-feet min. length. Wood posts shall be free of defects such as knots, splits, or gouges.
- No. 6 steel rebar or larger.
- ASTM A 120 steel pipe with a minimum diameter of 1-inch.
- U, T, L, or C shape steel posts with a minimum weight of 1.35 lbs./ft.
- Other steel posts having equivalent strength and bending resistance to the post sizes listed above.
- 11. Locate silt fences on contour as much as possible, except at the ends of the fence, where the fence shall be turned uphill such that the silt fence captures the runoff water and prevents water from flowing around the end of the fence.
- 12. If the fence must cross contours, with the exception of the ends of the fence, place gravel check dams perpendicular to the back of the fence to minimize concentrated flow and erosion. The slope of the fence line where contours must be crossed shall not be steeper than 3H:1V.
 - Gravel check dams shall be approximately 1-foot deep at the back of the fence. Gravel check dams shall be continued perpendicular to the fence at the same elevation until the top of the check dam intercepts the ground surface behind the fence.
 - Gravel check dams shall consist of crushed surfacing base course, gravel backfill for walls, or shoulder ballast. Gravel check dams shall be located every 10 feet along the fence where the fence must cross contours.

Maintenance Standards

- Repair any damage immediately.
- Intercept and convey all evident concentrated flows uphill of the silt fence to a sediment pond.
- Check the uphill side of the fence for signs of the fence clogging and acting as a barrier to flow and then causing channelization of flows parallel to the fence. If this occurs, replace the fence or remove the trapped sediment.
- Remove sediment deposits when the deposit reaches approximately one-third the height of the silt fence, or install a second silt fence.
- Replace filter fabric that has deteriorated due to ultraviolet breakdown.

	REV. NO	DATE	REVISED DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	Ck B
LIAM N. TAK					
shilling i					
Person Pe					
STONAL EN					

BMP C103: High Visibility Fence

Purpose

- Fencing is intended to:
- Restrict clearing to approved limits.
- 2. Prevent disturbance of sensitive areas, their buffers, and other areas required to be left undisturbed.
- 3. Limit construction traffic to designated construction entrances, exits, or internal roads.
- 4. Protect areas where marking with survey tape may not provide adequate protection.

Conditions of Use

To establish clearing limits plastic, fabric, or metal fence may be used:

- . At the boundary of sensitive areas, their buffers, and other areas required to be left uncleared.
- As necessary to control vehicle access to and on the site.

Design and Installation Specifications

High visibility plastic fence shall be composed of a high-density polyethylene material and shall be at least four feet in height. Posts for the fencing shall be steel or wood and placed every 6 feet on center (maximum) or as needed to ensure rigidity. The fencing shall be fastened to the post every six inches with a polyethylene tie. On long continuous lengths of fencing, a tension wire or rope shall be used as a top stringer to prevent sagging between posts. The fence color shall be high visibility orange. The fence tensile strength shall be 360 lbs./ft. using the ASTM D4595 testing method.

If appropriate install fabric silt fence in accordance with BMP C233: Silt Fence (p.367) to act as high visibility fence. Silt fence shall be at least 3 feet high and must be highly visible to meet the requirements of this BMP.

Metal fences shall be designed and installed according to the manufacturer's specifications.

Metal fences shall be at least 3 feet high and must be highly visible.

Fences shall not be wired or stapled to trees.

Maintenance Standards

If the fence has been damaged or visibility reduced, it shall be repaired or replaced immediately and visibility restored.



DESTROY ALL PRINTS BEARING EARLIER DATE	CKD. BY	APRD BY				LI RESIDENCE	
				Cannon		TESC DETAIL	c
						IESC DETAIL	
					ME	RCER ISLAND, WASH	IINGTON
				1050 Southwood Drive			
				San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863	DRAWN BY	DATE 3/14/2023	CA JOB NO. 220418
			CANNON. ALL DESIGNS AN	RUMENTS OF SERVICE AND ARE THE PROPERTY OF D_INFORMATION ON THESE DRAWINGS ARE FOR USE	CHECKED BY	SCALE	SHEET
				DJECT AND SHALL NOT BE USED OTHERWISE OR HE EXPRESSED WRITTEN PERMISSION OF CANNON.	KR KR	AS SHOWN	C9 OF 8

Drainage Pump Head Loss Design

PROJECT: DATE:	Li Residence 12/12/2022	
Q100=	= 0.05 cfs =	22.44 gpm
Elevation head pump elev.= discharge elev.= he=	= 300.94 High Leve = 312.46	I 300.94 Low Level 309.29 High Level 5.24
Friction head		
Darcy Weisbach Equati	on: h _f =f*[(L/D)*(V ² /2g)]	
	= 32.2 ft/s	
Re=	$Re = V^{*}(^{D}/_{y})$ v = 1.41*10 ⁻⁵ = 36,120	ref. pg 884, Pumping Station Design
€= [¢] / _D = f= hf=	= 0.0053333 = 0.044	ref pg 896, Pumping Station Design or CERN A-41 Apper
Fitting losses	5	
Bends K= # of bends= h _L = Expander	$h_{L} = (K^* V^{2)} / (2g)$ = 0.25 = 1 = 0.06 ft	bend losses per 90° bend
pump disch. dia.= carrier dia.= V1 V2	= 0.00 in = 0.00 in 0.00 ft/s	(enter zero if no expander) (enter zero if no expander) V=Q/A yes
h _L = Total b		$h_L = (V_1^2 - V_2^2)/2g$ (enter zero if no expander)
Total h _L =	·	
Total Pump H=	⊧ he+hf+hl	
H=	= 13.74 ft	

New and Replaced		Detention Pipe Length (ft)			Lowest Orifice Diameter (in) ⁽³⁾		Outlet Invert Orifice (ft)	Second Orifice Diameter (in)	
mpervious Surface Area (sf)	Detention Pipe Diameter (in)	B soils	C soils	B soils	C soils	B soils	C soils	B soils	C soils
	36"	30	22	0.5	0.5	2.2	2.0	0.5	0.8
500 to 1,000 sf	48"	18	11	0.5	0.5	3.3	3.2	0.9	0.8
	60"	11	7	0.5	0.5	4.2	3.4	0.5	0.6
	36"	66	43	0.5	0.5	2.2	2.3	0.9	1.4
1,001 to 2,000 sf	48"	34	23	0.5	0.5	3.2	3.3	0.9	1.2
1,001 (0 2,000 3)	60"	22	14	0.5	0.5	4.3	3.6	0.9	0.9
	36"	90	66	0.5	0.5	2.2	2.4	0.9	1.9
2,001 to 3,000 sf	48"	48	36	0.5	0.5	3.1	2.8	0.9	1.5
	60"	30	20	0.5	0.5	4.2	3.7	0.9	1.1
	36"	120	78	0.5	0.5	2.4	2.2	1.4	1.6
3,001 to 4,000 sf	48"	62	42	0.5	0.5	2.8	2.9	0.8	1.3
	60"	42	26	0.5	0.5	3.8	3.9	0.9	1.3
	36"	134	91	0.5	0.5	2.8	2.2	1.7	1.5
4,001 to 5,000 sf	48"	73	49	0.5	0.5	3.6	2.9	1.6	1.5
	60"	46	31	0.5	0.5	4.6	3.5	1.6	1.3
	36"	162	109	0.5	0.5	2.7	2.2	1.8	1.6
5,001 to 6,000 sf	48"	90	59	0.5	0.5	3.5	2.9	1.7	1.5
	60"	54	37	0.5	0.5	4.6	3.6	1.6	1.4
	36"	192	128	0.5	0.5	2.7	2.2	1.9	1.8
6,001 to 7,000 sf	48"	102	68	0.5	0.5	3.7	2.9	1.9	1.6
	60"	64	43	0.5	0.5	4.6	3.6	1.8	1.5
	36"	216	146	0.5	0.5	2.8	2.2	2.0	1.9
7,001 to 8,000 sf	48"	119	79	0.5	0.5	3.8	2.9	2.2	1.7
	60"	73	49	0.5	0.5	4.5	3.6	2.0	1.6
	36"	228	155	0.5	0.5	2.8	2.2	2.1	1.9
8,001 to 8,500 sf ⁽¹⁾	48"	124	84	0.5	0.5	3.7	2.9	1.9	1.8
	60"	77	53	0.5	0.5	4.6	3.6	2.0	1.6
	36"	NA ⁽¹⁾	164	0.5	0.5	NA ⁽¹⁾	2.2	NA ⁽¹⁾	1.9
8,501 to 9,000 sf	48"	NA ⁽¹⁾	89	0.5	0.5	NA ⁽¹⁾	2.9	NA ⁽¹⁾	1.9
	60"	NA ⁽¹⁾	55	0.5	0.5	NA ⁽¹⁾	3.6	NA ⁽¹⁾	1.7
	36"	NA ⁽¹⁾	174	0.5	0.5	NA ⁽¹⁾	2.2	NA ⁽¹⁾	2.1
9,001 to 9,500 sf ⁽²⁾	48"	NA ⁽¹⁾	94	0.5	0.5	NA ⁽¹⁾	2.9	NA ⁽¹⁾	2.0
5,001 (0 5,500 3)	40	NA ⁽¹⁾	57	0.5	0.5	NA ⁽¹⁾	2.5	NA ⁽¹⁾	1.7

Table 1

Notes:

• Minimum Requirement #7 (Flow Control) is required when the 100-year flow frequency causes a 0.15 cubic feet per second increase (when modeled in WWHM with a 15-minute timestep). Breakpoints shown in this table are based on a flat slope (0-5%). The 100-year flow frequency will need to be evaluated on a site-specific basis for projects on moderate (5-15%) or steep (> 15%) slopes.

Soil type to be determined by geotechnical analysis or soil map.

Sizing includes a Volume Correction Factor of 120%.

Upper bound contributing area used for sizing.

⁽¹⁾ On Type B soils, new plus replaced impervious surface areas exceeding 8,500 sf trigger Minimum Requirement #7 (Flow Control) ⁽²⁾ On Type C soils, new plus replaced impervious surface areas

exceeding 9,500 sf trigger Minimum Requirement #7 (Flow Control) ⁽³⁾ Minimum orifice diameter = 0.5 inches

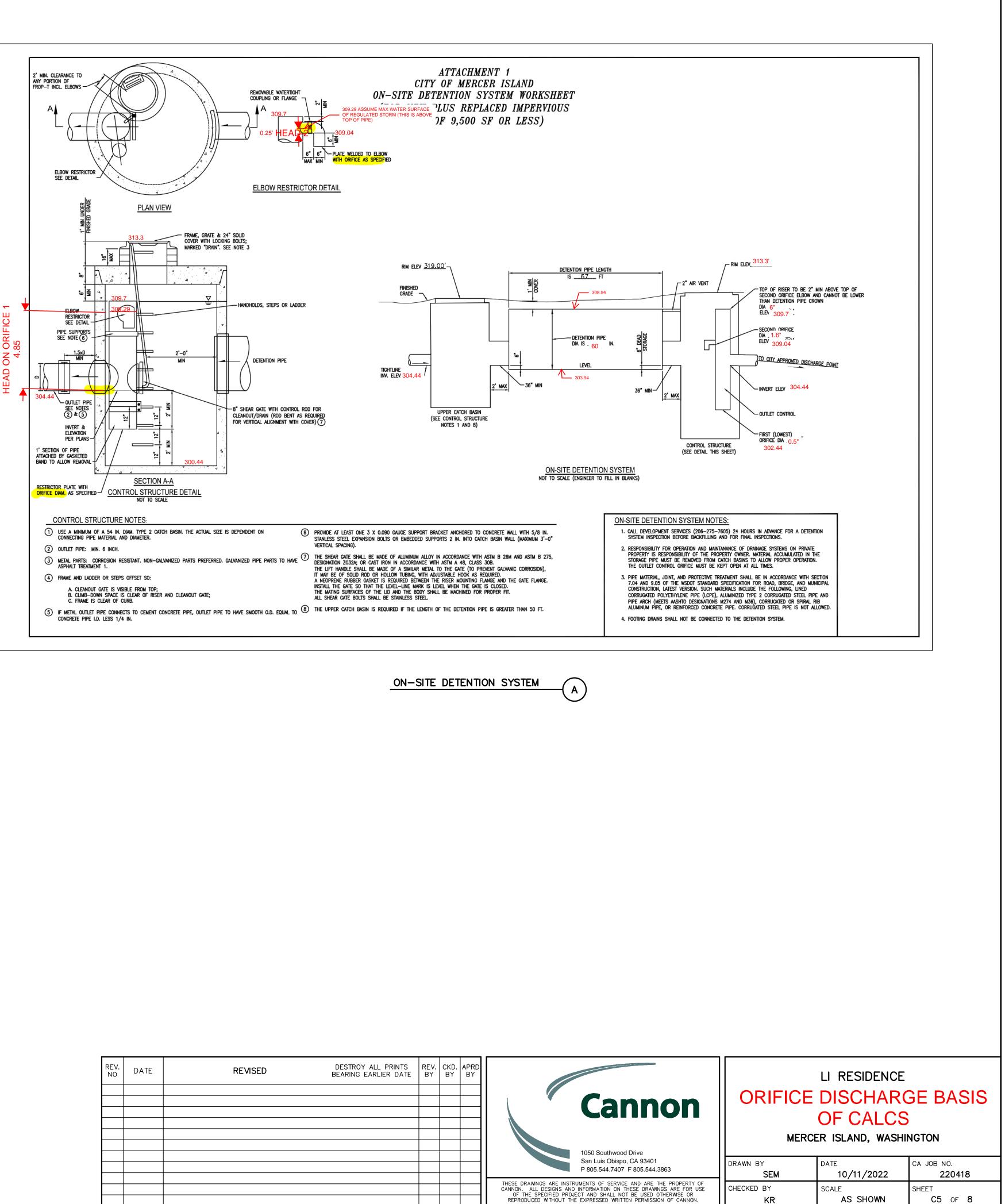
in = inch

ft = feet

sf = square feet

Basis of Sizing Assumptions: Sized per MR#5 in the Stormwater Management Manual for Puget Sound Basin (1992 Ecology Manual) SBUH, Type 1A, 24-hour hydrograph 2-year, 24-hour storm = 2 in; 10-year, 24-hour storm = 3 in; 100-year, 24-hour storm = 4 in Predeveloped = second growth forest (CN = 72 for Type B

soils, CN = 81 for Type C soils) Developed = impervious (CN = 98) 0.5 foot of sediment storage in detention pipe Overland slope = 5%



REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	CKI B1

Orifice Discharge

PROJECT: Li Residence DATE: 12/12/2022

Orifice 1		Input Calculated
Diameter:	0.5 inches	
Area	0.196 inches Sq 5.1.4	4.2 METHODS OF ANALYSIS
	0.001 SF	This section presents the methods and equations for design of control structure restrictor devices . Included are details for the design of orifices, rectangular sharp-crested weirs, v-notch weirs, sutro weirs, and overflow risers.
С	0.62	
А	0.001 SF	Orifices
g	32.2 ft/sec2	Flow through orifice plates in the standard tee section or turn-down elbow may be approximated by the general equation:
h	4.85	
		$Q = CA \sqrt{2gh} \qquad (5-4)$
Q1	0.015 CFS	where $Q = \text{flow (cfs)}$ C = coefficient of discharge (0.62 for plate orifice) A = area of orifice (sf) h = hydraulic head (ft)
Orifice 2		$g = \text{gravity}(32.2 \text{ fl/sec}^2)$
Diameter:	1.6 inches	Figure 5.1.4.D illustrates a simplified application of the orifice equation, assuming a water surface at the
Area	2.010 inches Sq	top of the riser and that the 2-year water surface represents the head in the outlet pipe.
	0.014 SF	
		FIGURE 5.1.4.D SIMPLE ORIFICE
С	0.62	1
C A	0.62 0.014 SF	∇ $Q = CA_1 \sqrt{2m} + CA_1 \sqrt{2m}$
	0.014 SF 32.2 ft/sec2	$ Q = CA_{b}\sqrt{2gh} + CA_{t}\sqrt{2gh} $ $ = C\sqrt{2g}(A_{b}\sqrt{h_{b}} + A_{t}\sqrt{h_{t}}) $
A	0.014 SF	$\overline{\Psi}_{h}$ $Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh}$
A g	0.014 SF 32.2 ft/sec2	$Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh}$ $= C\sqrt{2g} (A_b \sqrt{h_b} + A_t \sqrt{h_t})$ $h = DISTANCE FROM HYDRAULIC GRADE LINEb AT THE 2-YEAR FLOW OF THE OUTFLOW PIPE$
A g h	0.014 SF 32.2 ft/sec2 0.25 FT	$Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh}$ $= C\sqrt{2g} (A_b \sqrt{h_b} + A_t \sqrt{h_t})$ $h = DISTANCE FROM HYDRAULIC GRADE LINEb AT THE 2-YEAR FLOW OF THE OUTFLOW PIPE$
A g h Q1	0.014 SF 32.2 ft/sec2 0.25 FT	$\label{eq:constraint} \begin{array}{c} Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh} \\ = C \sqrt{2g} \left(A_b \sqrt{h_b} + A_t \sqrt{h_t} \right) \\ = D STANCE + ROM HYDRAULIC GRADE LINE \\ b ATTHE 2-YEAR FLOW OF THE OUTFLOW PIPE \\ TO THE OVERFLOW ELEVATION. \end{array}$
A g h Q1	0.014 SF 32.2 ft/sec2 0.25 FT 0.035	$Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh}$ $= C\sqrt{2g} (A_b \sqrt{h_b} + A_t \sqrt{h_t})$ $h = DISTANCE + FROM HYDRAULIC GRADE LINE b AT THE 2-YEAR FLOW OF THE OUTFLOW PIPE$
A g h Q1	0.014 SF 32.2 ft/sec2 0.25 FT 0.035 0.050 CFS	$\label{eq:constraint} \begin{array}{c} Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh} \\ = C \sqrt{2g} \left(A_b \sqrt{h_b} + A_t \sqrt{h_t} \right) \\ = D STANCE + ROM HYDRAULIC GRADE LINE \\ b ATTHE 2-YEAR FLOW OF THE OUTFLOW PIPE \\ TO THE OVERFLOW ELEVATION. \end{array}$
A g h Q1	0.014 SF 32.2 ft/sec2 0.25 FT 0.035 0.050 CFS	$Q = CA_b \sqrt{2gh} + CA_t \sqrt{2gh}$ $= C\sqrt{2g} (A_b \sqrt{h_b} + A_t \sqrt{h_t})$ $h^{e} DISTANCE FROM HYDRAULIC GRADE LINEb ATTANCE THE OVERFLOW OF THE OUTFLOW PIPETO THE OVERFLOW ELEVATION.$ The diameter of the orifice (b)

2021 Surface Water Design Manual

5-29

7/23/2021



RESIDENTIAL CAST IRON SUBMERSIBLE SUMP PUMPS MCIO SERIES

The Myers MCIO Series are rugged cast iron sump pumps featuring a vortex impeller that provides solids-handling capability up to 1/2" Product is available in 115 Volt single phase 60 Hz. Pump discharge is 1-1/2" NPT. 2-pole float switch for minimum 10" diameter sumps. Manual models also available.

APPLICATIONS

Residential basement sumps, dewatering, light effluent and water transfer.

FEATURES

• Easy Fit

2-pole switch design allows for operation in sump diameters of 10" or more.

- Reliable and Tough
 Rugged, oil-cooled motor provides continuous
 bearing lubrication and maximizes heat dissipation.
- Extended Service

Lubricated ball bearings and shaft seal for longer service life.

- **Eliminates Failures** Shaded pole motor eliminates failure-prone switches and relays.
- **Motor Protection** Long-life carbon/ceramic seal protects motor against water leakage.
- **Thermal Overload** Heat sensor overload protection with automatic reset when motor cools to safe operating temperature.
- No Drilling Required Anti-airlock hole in base reduces added labor.
- Easy Serviceability

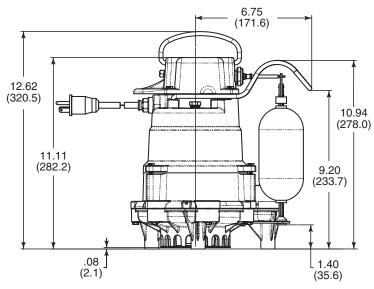
Pump, intake screen volute base, switch and power cord are serviceable.

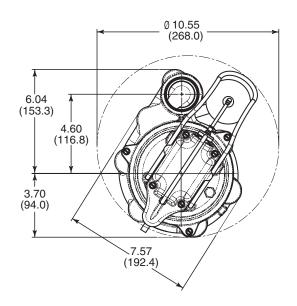
SPECIFICATIONS & PERFORMANCE

CAPACITIES	MCI033 - 48 GPM (182 LPM) , MCI050 - 62 GPM (235 LPM)
SHUT-OFF HEAD	24'(7.3 m)
FLOAT SWITCH ON/OFF DIMENSIONS	On: 9" (229 mm), Off: 4" (102 mm)
FLOAT SWITCH OPERATION	Automatic
SOLIDS HANDLING	1/2" (12.7 mm)
LIQUIDS HANDLING	Sump dewatering & effluent drainage
CONTINUOUS LIQUID TEMPERATURE	130°F (54°C)
MOTOR ELECTRICAL DATA	MCI033 - 98.A, MCI050 - 12A shaded pole with thermal overload protection, 115V, 1Ø, 60Hz, 1550 RPM
ACCEPTABLE PH RANGE	6-9
SHAFT SEAL	Carbon and ceramic
HOUSING/VOLUTE	Heavy cast iron
VOLUTE BASE	Fiberglass reinforced polypropylene or cast iron
IMPELLER	Fiberglass-reinforced Noryl® with threaded brass insert
EXTERIOR HARDWARE	Stainless steel
POWER CORD	10′ (3 m), 16/3, SJTW-A, SJTW 0r 20′(6m)
DISCHARGE, NPT	1-1/2″
MIN. BASIN DIAMETER	10"(25.4 cm)

DIMENSIONS

MCIO SERIES



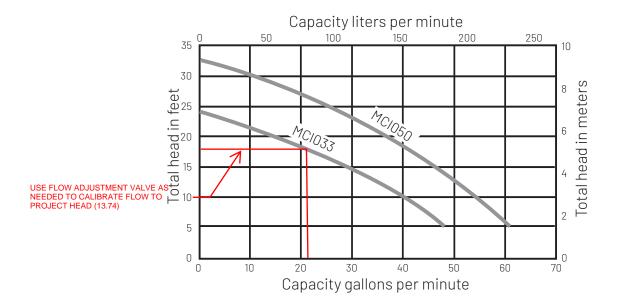


ORDERING INFORMATION

MODEL	CORD LENGTH	LOWER CASING VOLUTE Material	VOLTS	PHASE	AMPS	DISCHARGE	SWITCH
MCI033	10′(3.1 m)	Engineered Polymer	115	1	9.8	1-1/2" NPT	Vertical
MC103320	20′(6.1 m)	Engineered Polymer	115	1	9.8	1-1/2" NPT	Vertical
MCI033CI	10′ (3.1 m)	Cast Iron	115	1	9.8	1-1/2" NPT	Vertical
MCI050	10′ (3.1 m)	Engineered Polymer	115	1	12.0	1-1/2" NPT	Vertical
MCI05020	20′(6.1 m)	Engineered Polymer	115	1	12.0	1-1/2" NPT	Vertical
MCI050CI	10′ (3.1 m)	Cast Iron	115	1	12.0	1-1/2" NPT	Vertical

*Rest of pump is Cast Iron

PUMP PERFORMANCE





293 Wright Street, Delavan, WI 53115 USA Ph: 888.782.7483 Orders Fax: 800.426.9446 490 Pinebush Road Unit 4 Cambridge, Ontario N1T 0A5 Canada Ph: 800.363.7867 Orders Fax: 888.606.5484 For more details, visit pentair.com



All indicated Pentair trademarks and logos are property of Pentair. Third party registered and unregistered trademarks and logos are the property of their respective owners. Because we are continuously improving our products and services, Pentair reserves the right to change specifications without prior notice. Pentair is an equal opportunity employer.

Noryl® is a registered trademark of SABIC Global Technologies B.V.

©2021 Pentair. All Rights Reserved.